





April 24, 2020

PRIME AE Group, Inc. 3975 Fair Ridge Drive Suite 455N Fairfax, VA 22033

SoutheasternCollector Study



Town of Berryville Clarke County

Southeastern Collector Study

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Executive Summary

Overview

The Town of Berryville and Clarke County, VA have long envisioned a Southeastern Collector to encourage expansion of the Clarke County Business Park. To determine how to best incorporate the existing roadway network, connect across the Norfolk Southern Rail Line and finance the collector, the Town of Berryville and Clarke County commissioned the Southeastern Collector Transportation Study.

Four concepts for the Southeastern Collector were developed by Town and County Staff. There are three general alternatives to extend Jack Enders Boulevard and the Business Park. The fourth concept is a combination of two other concepts. The four concepts are below:

- Concept A. Extend Jack Enders Boulevard over NS railroad to US 340.
- 2. Concept B. Extend Jack Enders Boulevard to Smallwood Lane and improve Smallwood Lane to US 340.
- 3. Concept C. Extend Jack Enders Boulevard to US 340 and into Southern Potential Growth Area. This Concept is a combination of Concept A and D.
- Concept D. New Road in Southern Potential Growth Area without a connection to Smallwood Lane.

See exhibits in appendix.

Feasibility of New At-Grade Norfolk Southern Crossing

Although the Clarke County Business Park site plan shows an extension of Jack Enders Boulevard, the approval for a new-at-grade crossing is beyond the Town or County's control. As such, one of the objectives of this study is to determine the feasibility of a new crossing, and specifically of Concept A.

The feasibility of a new at-grade crossing depends on the answers to two key questions:

- 1) What would be necessary to obtain approval from Norfolk Southern for a new crossing?
- 2) Is the Jack Enders Boulevard extension the most optimal location for a new collector in the southeastern growth area?



Figure ES-1:

Southeastern Collector Location

Berryville Area and

Multiple conversations were held with Norfolk Southern staff. New, at grade crossings are not only highly discouraged, but also discouraged by Virginia State Statute¹. Two hypothetical scenarios were discussed with Norfolk Southern Staff to gage the feasibility of a new crossing. First, what if the Town closed two or more existing at grade crossings? Second, would Norfolk Southern be amenable if the Town created a new siding and rail related commercial area?

The Norfolk Southern staff were <u>not</u> amenable to these hypothetical scenarios. Norfolk Southern Staff cited Virginia State Statute, and current efforts to eliminate at grade crossings. Furthermore, the proposed location of the new at grade crossing for Concept A is located on a curve and has sight distance limitations for southbound trains and would be disruptive to local residents. Although Norfolk Southern staff were careful not to formally reject the proposed at grade crossing, they were very clear

Concept A

Figure ES-2: Concept A – Jack Enders Blvd

that it was not a realistic a concept².

To obtain approval of a new at grade crossing, Norfolk Southern requires a Concept Package be submitted³. Subsequent plans are normally reviewed by Norfolk Southern at the requester's expense. It is likely that the Town would need to enter into a Preliminary Engineering Agreement with Norfolk Southern and provide compensation to Norfolk Southern to continue applying the new at grade crossing.

The coordination with Norfolk Southern, research and analysis concluded:

- The location of the new crossing for Concept
 A is not acceptable to Norfolk Southern. Even
 extraordinary efforts by the Town and County will be insufficient to overcome the general aversion
 to new at grade crossings, and especially in the proposed location.
- 2. The Town and County do not have existing crossings that can be eliminated to continue a dialog with Norfolk Southern. Nor does the Town and County have plans for major investment in a rail facility to make the new crossing part of a larger package.
- 3. To further pursue the new crossing for Concept A will require developing a Concept Package, and possibly enter into a Preliminary Engineering agreement with Norfolk Southern, and incur the expense of Norfolk Southern engineering reviews.
- 4. The location of Concept A is not superior to the other options. Concepts B and D create more developable property than Concept A. Concepts B and D have less wetland impacts than Concept A. Concept A's only advantage is lower construction cost.

³ Norfolk Southern Railway Company, Public Projects Manual, Rev 2; Section 4 page 3.



¹ Va. Code Ann. § 56-363 (1996).

² Teleconference held June 19, 2019. Attendees from Norfolk Southern: Mr. Roger Bennett (NS Corp) and Mr. Scott Overbey (NS Corp).

As such, the Study Team recommended that the Town and County discontinue consideration of Concept A as the proposed Southeastern Collector. Concept C is a combination of Concept A and Concept D; as such this concept is also eliminated.

Final Concepts Selected for Study

Concept B and Concept D were selected for further study. Within both concepts are two variations, resulting in four Concepts, shown in Figure ES-2 with a typical road profile in Figure ES-4. The northern Terminus is shown in Figure ES-5

- 1. Concept B1- Extend Jack Enders Boulevard into the Smallwood Property, perpendicular to Craig's Run and onto Smallwood Lane. Upgrade Smallwood Lane and Smallwood Lane's existing at grade crossing of Norfolk Southern RR.
- 2. Concept B2 Similar to B1, however the roadway will run further east to take advantage of existing Smallwood Lane.
- 3. Concept D1 Identical to B1, without the connection to Smallwood Lane. This alternative will not provide a crossing of Norfolk Southern for the expanded Business Park.
- 4. Concept D2 Identical to B2, without the connection to Smallwood Lane. This alternative will not provide a crossing of Norfolk Southern for the expanded Business Park

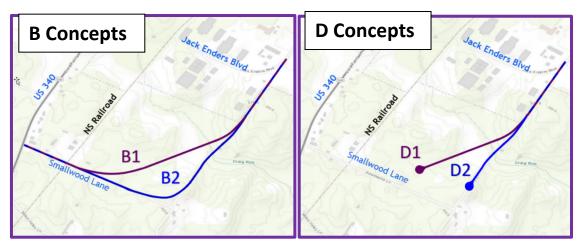


Figure ES-3: Concepts for Southeastern Collector

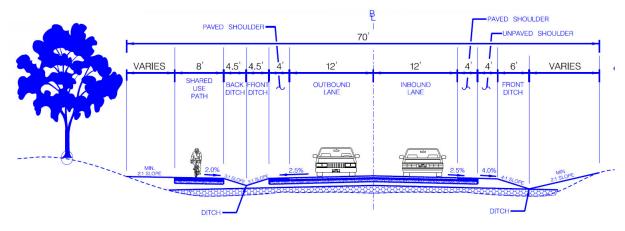


Figure ES-4: Typical Section of Southeastern Collector.





Figure ES-5: Southern Collector Northern Terminus

Changes in Travel Patterns

A microsimulation model of the Town was used to determine the changes in traffic patterns and changes in Level of Service at key intersections. The Land Use and Traffic Analysis found the following:

- > The development associated with the Collector roadway is projected to be 500,000 square feet (SF) of light industrial. This development is projected to generate 340 vph *from* the area and 110 vph *to* the area (PM peak hour).
- The downtown Berryville Main Street Intersections are projected to have modest increases in traffic volume and intersection delay.
- Additional traffic from Concept D is expected to use local streets to avoid the center of Berryville. With Concept D nearly 200 vehicles during the PM peak hour are projected to divert away from the intersection of US 340 and Main Street.
- Concept B creates a route to bypass downtown Berryville. The model projects 130 vph to use Concept B as a bypass under normal conditions (PM peak hour).
- The traffic at the Jack Enders Boulevard / Main Street intersection will increase by 494 vph, over 60%, with Concept D. Current total volume is 812 vph.
- ➤ With Concept D additional capacity is needed at Jack Enders Boulevard and Main Street. A new signalized intersection with a dedicated westbound left turn lane and eastbound right turn lane will be necessary.
- Initially the negative impacts from Concept D will be manageable. A new signal at Jack Enders Boulevard will not be necessary until development is well underway.
- > Traffic on Smallwood Lane will increase from 10 vph to 533 vph with Concept B. This will create the need for improvements to US 340 and, eventually, a new signal.



Environmental Impacts

Natural Environment – Both Concepts are largely located in farmland, as such impacts to habitat, forests, and endangered species are estimated to be minimal. The impacts will be limited to Craig's Run, an intermittent stream which is surrounded by wetlands, and a freshwater forested wetland.

The B1 and D1 Concepts have the least impacts to wetlands. They were developed to have a clean, direct perpendicular crossing of Craig's Run. Nonetheless the B2 and D2 Concepts are still estimated to impact only 1 acre of wetlands.

Historical Resources – The Concepts are not located near any state or federal registered historic properties. There are three structures impacted by Concepts B2 and D2 along Lindey Lane. These structures were determined by Clarke County to have some historic significance.

Project Costs

Table ES-1 summarizes the cost of each Concept. Concept D1 and D2 are less costly by virtue of terminating before Smallwood Lane.

| Table ES-1 Comparative Costs (\$ thousands) | | | | |
|---|---------|---------|---------|---------|
| | B1 | B2 | D1 | D2 |
| Construction Cost | | | | |
| Mainline | \$8,130 | \$7,950 | \$5,280 | \$4,820 |
| Intersection Improvements | \$560 | \$560 | \$430 | \$430 |
| New Traffic Signals | \$600 | \$600 | \$540 | \$540 |
| At Grade Railroad Crossing | \$160 | \$160 | \$0 | \$0 |
| Subtotal Construction Cost ⁴ | \$9,460 | \$9,280 | \$6,250 | \$5,790 |
| ROW Cost | \$100 | \$100 | \$60 | \$70 |
| Total Cost | \$9,560 | \$9,380 | \$6,310 | \$5,870 |

⁴ Construction cost per mile: B1 - \$9,080; B2 - \$8,670; D1 - \$10,310; D2 - \$10,200



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Summary – Advantages and Disadvantages of Each Concept

Concept B1 – This Concept is the most expensive but provides the most benefit. The alignment splits the Smallwood Property providing a central roadway for the new business park. The additional connection to US 340 aids the flow of traffic from the new and existing business park.

The alignment also minimizes the impact to Craig's Run and surrounding wetlands. However, it is the costliest Concept with an estimated cost of \$9.6 Million

Concept B2 – This Concept is a variation of B1 and uses existing Lindey Lane. Compared with B1 it is located on the edge of the Smallwood Property, and has a less direct crossing of Craig's Run and increased environmental impacts. However, this Concept has the same traffic benefits as B1 and a slightly lower cost of \$9.4 Million.

Concept D1 – This Concept follows the B1 alignment; however, it does not provide a new connection to US 340. As such it results in additional traffic through downtown Berryville and will require an upgrade to the intersection of Jack Enders Boulevard and East Main Street. The cost is lower than either of the "B" Concepts at \$6.3 Million.

Concept D2 — As with B2 this Concept is on the edge of the Smallwood Property and will not be a central roadway for the new business park. It also has a less direct crossing of Craig's Run, which increases the environmental impacts to Craig's Run. However, this Concept has the lowest cost of \$5.9 Million.

| Table ES-2 Summary of Costs and Benefits | | | | | |
|--|---|--|---|---|--|
| | B1 | B2 | D1 | D2 | |
| Land Use | Bisects Smallwood Property | Eastern Edge of Smallwood Property not optimal | Bisects Smallwood Property | Eastern Edge of Smallwood Property not optimal | |
| Environmental | Minimal impact to Craig's Run | Not as environmentally preferred crossing of Craig's run | Minimal impact to Craig's Run | Not as environmentally preferred crossing of Craig's run | |
| Traffic Flow | Improves traffic flow throughout | Improves traffic flow throughout | Large increase on Jack Enders Boulevard, traffic diversions onto Berryville streets | Large increase on Jack Enders Boulevard, traffic diversions onto Berryville streets | |
| Implementation | More \$\$, includes new connection at US 340. | More \$\$, includes new connection at US 340. | Can be expanded after initial phase. | Can be expanded after initial phase | |
| Total Cost | \$9,560 | \$9,380 | \$6,310 | \$5,870 | |



Conclusions, Implementation and Recommendations

Concept B1 best meet the needs of the Town and County. The Concept provides an upgraded crossing of the Norfolk Southern Railroad, improved traffic flow in and around Berryville and best promotes future development in the Southern Growth Area. The estimated cost is \$9.6 Million. A variety of funding sources are appropriate for this project as listed in table ES-3.

Planning, funding and building the new roadway will require a multi-pronged and multi-phased approach.

First, the Southern Growth Area, primarily on the Smallwood property, will need to be planned. With a vision of how this property will be redeveloped the Town and County can add this vision to the Comprehensive Plan and update the zoning. These initial planning steps will allow the roadway to compete for Smart SCALE funding and open the potential for grants and investment from private developers.

| Table ES-3 Funding Sources | | | |
|--|---|--|--|
| Transportation Funds | Brief description | | |
| Smart SCALE | Primary source for roadway funding. Smart SCALE Prioritizes projects for use of transportation State and Federal funds. | | |
| Revenue Sharing | VDOT program, provides a 50% match for qualifying projects. Other funds can be used for 50% match | | |
| Public Private Partnerships | The Public Private Transportation Act enables local governments to partner with private entities to build roadways. | | |
| State Grant Funder | | | |
| Community Development Block Grant | Based upon demographics and community need | | |
| FEMA flood protection policies and | Flood education, policy enforcement, construction | | |
| regional planning | standard updates, ordinance review | | |
| VDOT SRTS | Safe routes to schools, walking trails, bike trails | | |
| Go Virginia, Growth and Opportunity | Tech sector partnerships to develop economy in rural areas | | |
| | | | |
| Federal Grant Program | | | |
| USDA/NRCS Watershed Protection grants | For water quality, water supply protection, habitat | | |
| US Forest Service Land and Water Conservation Fund | Purchase land for permanent protection | | |
| TIGER/Build grants | Public transportation program 20% for urban areas | | |
| Redismart, department of Energy | For smart grid design implementation | | |
| INFRA program | Transportation that promotes economic vitality, accountability along freight highway | | |



Second, the Town and County need to actively pursue funds from State and Federal grants and investment from developers. An extension of Jack Enders Boulevard into the Smallwood Property, like Concept D1, will encourage some initial development on the Property. With this initial development it will be easier to attract other users or developers to the property. The site will be able to generate revenue and provide the funds to finish the Collector with either Smart SCALE or Revenue Sharing.

With a marketable vision the site and roadway will be a candidate for a Virginia Public Private Partnership (P3). This program will allow the Town and County to contract the development of the site. A private entity will assume much of the funding and risk, and in turn receive either future revenues or profits from the site.

Smart SCALE is the dominant program for allocating State and Federal transportation funds. In rural areas, the largest source of points is Economic Development. The Southern Growth Area will need to be planned for the Southeastern Collector to gain Economic Development points. Without Smart SCALE, the Town and County can also receive a 50% match using the Revenue Sharing program. With Concept B1, the Town and County will need to raise \$ 4.8 Million (one half of \$9.6 M). Other grants and private money can be used for this match.

Concept B1 could be implemented in four phases:

- I. Extend Jack Enders Boulevard into the Smallwood property to stimulate initial development.
- II. As development progresses, improve the Jack Enders Boulevard / Main Street Intersection.
- III. As development approaches 75% of buildout, extend the Connector to US 340.
- IV. When traffic warrants, add a signal at the intersection of Smallwood Lane and US 340.



Figure ES-6: Timing of Connector Construction

The timing of the phases will depend on the type of development and corresponding number of trips generated. When development reaches 25% of full buildout approximately 125 thousand SF of development will have occurred. For a typical industrial park this will create an additional 870 new daily trips and 107 new peak hour trips. This typical scenario is not likely to trigger the need for roadway improvements, however if the new development has greater than typical trip generation or a high number of trucks then roadway improvements may be necessary.



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When the development reaches 50% of buildout and 250 thousand SF of development, improvements to Jack Enders Boulevard will be necessary. At this point it will also be necessary to gain environmental approvals and begin design of the Connector. By 75% of buildout and 375 thousand SF of development it will be time to complete the Connector to US 340. The final improvement is a signal at US 340 and Smallwood Lane. This improvement should be implemented when conditions warrant, likely after 75% development.

The Southeastern Collector and the accompanying development will create many benefits for the Town and County. We recommend that the Town and County select Concept B1 and begin the visioning and planning for the associated development.



1. Existing Conditions

1.1 Background

The Town of Berryville and Clarke County, Virginia have long envisioned a Southeastern Collector to encourage development and provide connectivity. To determine how to best incorporate the existing roadways, connect across the Norfolk Southern Rail Line and to finance the collector the Town and County commissioned the Southeastern Collector Transportation Study.

The Town of Berryville Area Plan proposes to expand the existing Business Park along Jack Enders Boulevard into an area designated as the Southern Potential Future Growth Area. To support development a connection between Buckmarsh Street (US 340) and Main Street (VA Business 7) is necessary.

The default location for the Southeastern Collector has been along Jack Enders Boulevard, and across the Norfolk Southern railroad on a new at-grade railroad crossing. This connection will provide the needed connection from US 340 to VA Route 7 Business, serve the existing businesses along Jack Enders Boulevard and further open the Southern Potentail Growth Area across the Norfolk Southern rail line.

However, new at grade rail crossings are discouraged by Virginia Statute⁵, and are not generally approved by Norfolk Southern without closing at least one existing at-grade crossing in the vicinity and without the addition of a rail terminal or siding.

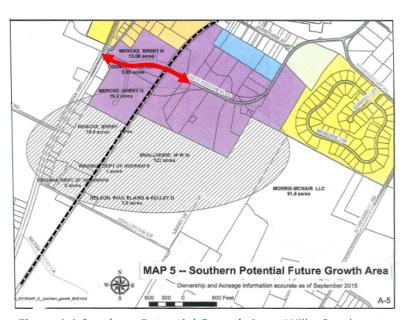


Figure 1.1 Southern Potential Growth Area. Will a Southeastern Collector be effective without a new or improved crossing of the railroad? *Source of map:* 2015 Berryville Area Plan

Other additions to the Jack Enders Boulevard - Clarke County Business Park area could be made to the south into the Smallwood property. The Town and County identified four concepts:

⁵ § 56-363. <u>Crossing of a railroad or public highway by another railroad; crossing of a railroad by a public highway</u>. This statute states that crossings "shall, wherever reasonably practicable, pass above or below the existing facility".



Concept A – Extend Jack Enders Boulevard from current terminus west across Norfolk Southern Railroad with new at-grade crossing to connection point near the intersection of South Church Street and US 340.

Concept B – Extend Jack Enders Boulevard to Smallwood Lane then west to US 340.

Concept C – Extend Jack Enders Boulevard to US 340 as in Concept A and extend Jack Enders Boulevard south into the Smallwood property.

Concept D — Extend Jack Enders Boulevard south (as in Concept C) without a new connection across NS Rail Line.

The Town and County further established the following goals for the Study⁶:

- Identify the feasibility and cost of each concept including total cost of developing a new atgrade crossing (Concepts A and C) or improving an existing at-grade rail crossing (Concept B).
- Determine any required improvements along the corridor to maintain acceptable Levels of Service (LOS) including but not limited to the need for new signalized intersections, turn lanes, crosswalks, and drainage improvements.
- Project how implementation of each Concept will impact traffic patterns along all corridors to be examined. This would help to determine the amount and nature of current traffic that would use Jack Enders Boulevard as a bypass to avoid downtown Berryville.

The purpose of this Section is to establish the safety, traffic, environmental, land use and business conditions in the Berryville area. These conditions will be the basis for further defining the Collector concepts and determining their benefits, costs and impacts.

1.2 Existing Traffic Conditions

Berryville is in the center of Clarke County, VA at the intersection of VA Route 7 and US Route 340. Both corridors are long-standing transportation corridors that were known in the 18th century as the Winchester Turnpike and Charles Town Road, respectively. Although the major highway Interstate corridors in this region are I-66 and I-81, both VA Route 7 and US Route 340 provide supplementary service.

Traffic conditions in the Town of Berryville are governed by Main Street (VA 7 Business) and US Route 340. Other than external traffic on VA Route 7, the bulk of Town trips pass on either Main Street or US Route 340. The Town's system of collector roadways (identified as Jack Enders Boulevard, Mosby Boulevard, Hermitage Boulevard, and Fairfax Street) depend on Main Street and US Route 340 to function with acceptable levels of service.

In Town, both roadways are urban roadways with frequent driveways, on-street parking and limited roadway width. Main Street serves local uses and helps create the historic small town feel that is valued by residents and visitors. It is not consistent with the setting for these roadways to pass large volumes of traffic. As such, capacity and traffic flow can be easily disrupted by large trucks or even minor incidents.

⁶ Berryville Town Council & Clark County Board of Supervisors Joint Meeting 11 December 2018, Attachment 1 Scope of Transportation Study, page 3.



Traffic Volumes and Levels of Service

Traffic conditions are generally acceptable in the Town. Figure 1.2 shows Average Annual Daily Traffic from the VDOT database⁷; Figure 1.3 shows turning movements for the Town's key intersection at US 340 and Main Street. On the following page, Table 1.1 shows the calculated LOS based on traffic counts taken at the three intersections shown.

Figure 1.2 shows that the Route 7 traffic is separate from Berryville Town traffic. The large volumes on Route 7 have a high Directional "D" value, indicating that the traffic is predominantly eastbound in the morning and westbound in the evening. The other roadways in Figure 1.2 have modest "D" values and lower volumes. Some roadways have existing limitations. For instance, through trucks are prohibited on Route 7 Business (Main Street). US 340 has moderate truck traffic and north-south truck traffic through Town is unavoidable. US 340 in Berryville has 8% truck traffic. There are also restrictions of Mosby and Hemitage.

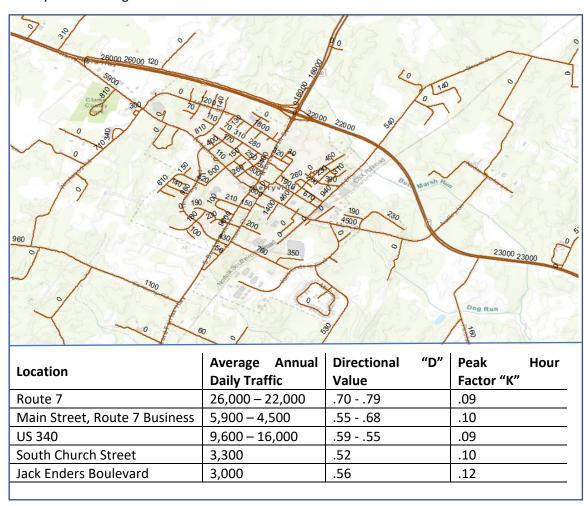


Figure 1.2 Average Daily Traffic Volumes and Key Traffic Characteristics

⁸ Ibid.



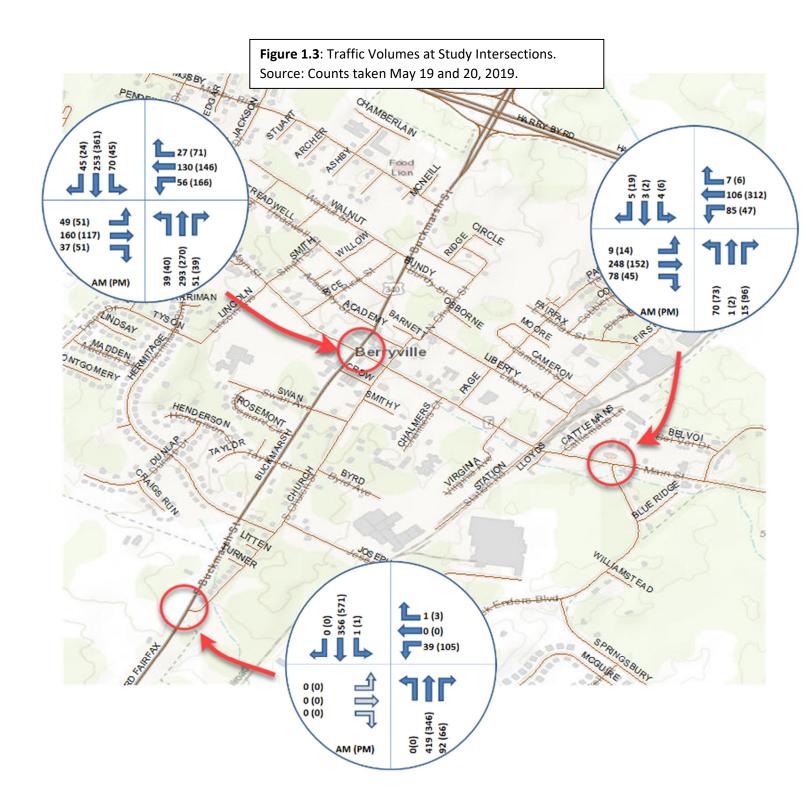
⁷ VDOT, 2018 Daily Traffic Volume Estimates, Special Locality Report 168 Berryville.

Traffic Counts were conducted on May 21, 2019 for three intersections. Key traffic characteristics are shown in Table 1.1. Figure 1.3 shows turning movement counts by movement.

| Table 1.1: Intersection Turning Movement County Summary | | | | |
|---|----------------------------------|---------------------------|--|--|
| Intersection | Peak Period AM / PM | Total Intersection Volume | | |
| Main Street at Jack Enders Blvd | 7:00 – 8:00 AM 3:30 – 4:15 PM | 731 AM 775 PM | | |
| US 340 at Main Street | 7:15 – 8:15 AM 4:15 – 5:15 PM | 1,210 AM 1,381 PM | | |
| US 340 at Church Street | 7:15 – 8:15 AM 4:15 – 5:15 PM | 1,009 AM 1,092 PM | | |

Town traffic is largely local traffic with a destination or origin in the Town or immediate area. The exception is external traffic that is passing through the Town on US 340. The largest movement at the intersection of US 340 and Main Street is the US 340 through (i.e. north-south) traffic. North-south volumes at Main Street are 50% greater than east-west volumes, as such it is likely that 1/4 to 1/3 of the north-south peak hour traffic through the center of town is through traffic. (See Figure 1.2 and Table 1.2.)

| Table 1. 2: Intersection Volumes and LOS by Approach | | | | | |
|--|------------|----------------|---------------|---------|--|
| Intersection | Approach | Volume AM (PM) | Delay Sec | LOS | |
| US 340 - Main | | | | | |
| Street | | | | | |
| | Northbound | 383 (349) | 22.4 / (21.0) | C (C) | |
| | Southbound | 368 (430) | 18.8 / (23.9) | B (C) | |
| | Eastbound | 246 (219) | 28.3 / (28.5) | C (C) | |
| | Westbound | 213 (383) | 25.5 / (27.8) | C (C) | |
| Main Street - | | | | | |
| Jack Enders Blvd | | | | | |
| | Northbound | 186 (171) | 13.6 / (13.6) | B / (B) | |
| | Southbound | 12 (27) | 14.0 / (14.2) | B / (B) | |
| | Eastbound | 335 (212) | 0.2 / (0.5) | A / (A) | |
| | Westbound | 198 (365) | 3.5 / (1.0) | A / (A) | |
| US 340 - Church | | | | | |
| Street | | | | | |
| | Northbound | 611 (412) | 0.0 / (0.0) | NA | |
| | Southbound | 357 (572) | 0.0 / (0.0) | NA | |
| | Westbound | 40 (108) | 19.5 / (29.6) | C / (D) | |



Crash History

Vehicular crashes in Berryville are typical for the roadways and volume in the Town. Analysis of 122 crashes over 3 years on US 340 and Main Street (RT 7 Business) and Jack Enders Boulevard reveal that most crashes are at intersections or driveways. In addition, non-injury crashes (i.e. Property Damage Only) dominate.

Berryville crashes are less severe than area crashes in general. This is likely due to higher speeds on rural roadways outside the built-up towns and cities. As shown in table 1.3, Berryville injury crashes on primary routes consist of only 9% of total crashes, as compared to 35% for Clarke County and 31% for the greater Northern Shenandoah Valley Area.

| Table 1.3: Three-year (2016-2018) Crash Percentages on Primary Routes ⁹ | | | | |
|--|-----------------|---------------|--------------|--|
| Crash Type | Berryville Area | Clarke County | NSVRC Area. | |
| Fatality | 0 / 0% | 7 / 1% | 51 / 1% | |
| Injury A | 1/1% | 46 / 8% | 250 / 5% | |
| Injury B | 5 / 7% | 128 / 22% | 1,176 / 22% | |
| Injury C | 1/1% | 17 / 3% | 149 / 3% | |
| Property Damage Only (PDO) | 68 / 91% | 373 / 65% | 3,678 / 69% | |
| Total | 75 / 100% | 571 / 100% | 5,304 / 100% | |

Note: All comparisons with Clarke County and Northern Shenandoah Valley Regional Commission (NSVRC) area are for crashes on Primary Routes. The NSVRC area consists of the Counties of Clarke, Frederick, Shenandoah, Warren and Page and the towns and cities located within these areas.

Injury A – Evacuation for medical treatment, Injury B – Injury treated on site. Injury C – Complaint, no visible injury.

Of the crashes analyzed, 70% of the crashes can be described into 4 types. The top two crash types in the area were 1) failure to obey a signal or properly yield the right-of-way at an intersection, and 2) rear-end in heavy traffic. Other types of rear-end and maneuvers in or out of driveways were the next most prevalent types of crashes. Below is a breakdown of the crashes analyzed:

Failure to Obey Signal or Yield Right-of-Way at Intersection: 27 (22%)

Rear End in Heavy Traffic: 19 (22%)

• Rear End at Signal or Other Location: 16 (13%)

Turn In/Out of Driveway: 16 (13%)

Deer Crashes: 7 (6%)Parking Related: 5 (4%)

• Run off Road (Not Asleep): 5 (4%)

• Fell Asleep at Wheel: 4 (3%)

Other (13%)

⁹ VDOT, Crash Analysis Tool. VDOT receives crash data via DMV TREDS System.



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The following segments and intersections had the largest numbers of crashes; therefore, trends could be drawn for these specific locations:

| Table 1.4: Summary of Crashes ¹⁰ | | | | |
|---|------------------|--|---|---|
| Segment Description | Total Crashes | Severity | Primary Trend | Notable Crashes |
| Main Street @ US 340 | 7 | 6 PDO, 1 Injury | Rear ends (57%) | Truck turning right from EB Main to SB Buckmarsh struck pole. Distracted Truck Rear ended vehicle at light. |
| Main Street @ N. Church St | 12 | 11 PDO, 1 Injury | Parking related (25%) | Pedestrian hit by left turn while in crosswalk. |
| Main Street between N. Church and Route 7 | 9 | 7 PDO, 2 Injury | Parking related (33%); driver fell asleep (33%) | Confused driver turned left onto railroad tracks and got stuck. |
| Main Street @ Jack Enders Blvd | 3 | 3 PDO | Turning vehicle accidents (100%) | Right turning truck from Jack Enders Boulevard clipped vehicle in left turn lane. |
| Main Street @ Route 7 | 14 | 14 PDO | Rear ends due to heavy traffic (57%) | Most crashes are a result of rear ends in heavy traffic. |
| Rt. 7 Between US 340 and Parshall Rd | 25 | 16 PDO, 5 Injury, 4 Serious Injury | Rear ends due to heavy traffic (40%) | Most crashes are a result of rear ends in heavy traffic. |

Truck Involved Crash Trends: In total across the studies roadway segments, there were 15 (12%) large truck related crashes of the 122 crashes analyzed. Truck traffic is less than 9% on all the roadways. On US 340 from Main Street to RT 7 (Harry Byrd Highway) 7 of the 33 crashes (21%) of the crashes involved heavy trucks.

Rail Crossing Crashes: There was one rail crossing involved crash. The crash did not involve the train. The crash was due to driver confusion for mistaking the tracks as a road. The vehicle turned from west bound E. Main St. south onto the tracks that cross E. Main St., lodging the vehicle on top of the tracks. The vehicle was stuck and needed to be towed off the tracks. There were no other rail-related vehicle crashes reported over the period of 2016-2018.

 $^{^{10}}$ VDOT, Crash Analysis Tool. VDOT receives crash data via DMV TREDS System.



1.3. Natural and Historic Resources

Natural Resources

Berryville is located in the Shenandoah Valley in the Great Appalachian Valley. The area is punctuated by rolling hills, valleys and streams. The area is largely Karst topography, which is characterized by underground drainage systems due to the solubility of the underlying limestone.

In the study area, the most prominent feature affecting the location of a future roadway and development is Craig's Run. This stream if surrounded by wetlands, and a freshwater forested wetland. Craig's Run is listed on the EPA 303d list for impaired waters in Virginia for E-coli from NPS agricultural runoff. The wetlands fall under section 404 of the Clean Water Act requiring delineation and permitting for any impacts.

Forestland in the area will also have to be identified and quantified for its natural resource value. The Karst geology is prone to sinkholes and seeps, making it an unpredictable region to build upon, though not impossible. The soil region is the Northern Mountains and Piedmont of the Eastern Mountains and Piedmont and this study area is dominated by Poplimento-Webbtown soils.

According to the Department of the Interior, several potential threatened/endangered species may exist in the study area including: *Myotis sodalis* (Indiana bat), *Myotis septentrionalis* (Northern Longeared bat), and *Antrolana lira* (Crustacean or unpigmented troglobite). There may also be a *Bartramia longicauda*, (Upland sandpiper) in the region though no sightings have been identified since 2014. All these resources will require further study prior to any development activity.

A map showing area Natural Resources is in Figure 1.4.

Historic Resources

Historic resources in the area include the Josephine City Historic District which includes Josephine Street, the Josephine Community Museum and the Milton Valley Cemetery. This district is listed in the National Registry, and the Virginia Cultural Heritage Listing and Virginia Department of Historic Resources. These areas are shown in Figure 1.4.

Resources in Figure 1.4 marked with stars (clusters of local historic structures) are not officially protected; however, they are locally significant remnants of local history.



Conclusions:

- Crossing of Craig's Run will require wetland/forest study and Army Corps permits after jurisdictional determination by Virginia Department of Environmental Quality
- Forest cover areas will require further study
- All alternatives will require a Section 7 review for the bat and the crustacean (this is normal for construction in this area)
- Milton Valley Farm is a permanent easement that prohibits disturbance and may also require
 a scenic buffer.

Natural Resources Investigated in Study Area

ADC National Map Grid

Big Tree National registration site

Cornell lab of ornithology, https://www.allaboutbirds.org/guide/Upland_Sandpiper/maps-sightings

DEQ, Virginia Department of Environmental Quality

DCR, Virginia Department of Conservation and Recreation

FEMA flood maps

Geology review

Municipal Water/Sewer District map

Municipal Zoning map

National Park Service site maps

National Wetland Map

Regional contour maps

Soil Region summary

Shenandoah River PCB TMDL data (VA DEQ Valley Regional Office)

State Forest Cover map

Sub Watershed, Dog Run, Craig Run, 303D list of impaired waters

USDA/NRCS Soil survey

US Department of the Interior, IPAC, USFWS Species search

Virginia Outdoors Fund regional identification of Conservation Easements

Watershed HUC data EPA, Potomac Shenandoah Watershed

Cultural Structure Review in Study Area

Clarke County Conservation Easement Map

Clarke County Government Historic District Driving and Walking Tour Map

Clarke County Historical Association

Clarke County survey of structures (discussion with County Point of Contact)

Historic topo graphic maps

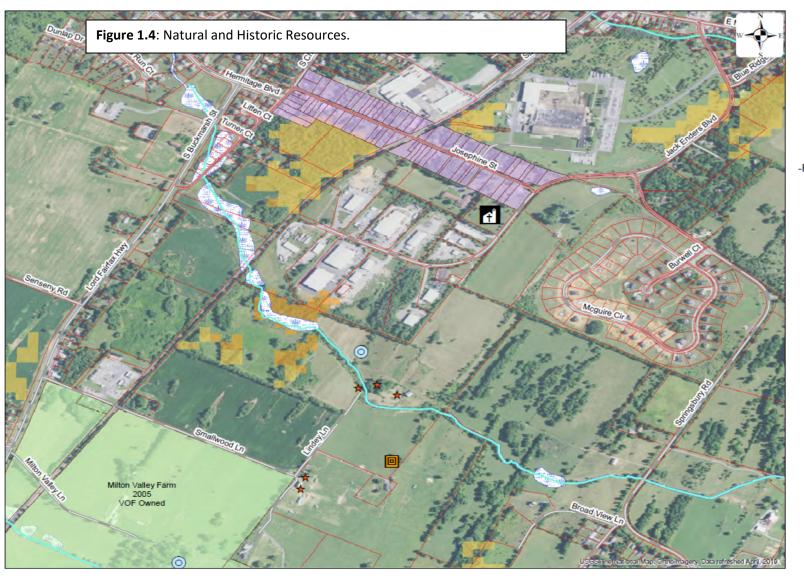
Josephine School Community Museum

National Trust for Historic Preservation

Scenic Byway setback area

Virginia Department of Historic Resources





Primary Soil Group: Poplimento Karst Geology

-Potential T/E species habitat in caves
-Potential sinkholes
-Springs
-Historic Structures
-Conservation Easements
-Cemetery - Milton Valley



1.4 Land Use

Clarke County is a scenic rural County that seeks to focus growth that will preserve its rural character. Future commercial, retail, subdivision residential and industrial growth is planned to occur in the Town of Berryville. Similarly, Berryville is known as an historic small Town with a quaint downtown. The Town's land use plan seeks to maintain this character and provide for a multitude of land uses through careful planning and execution. As stated in the 2015 Berryville Area Plan:

"The overriding purpose of the Plan is to encourage development of a safe, healthy, and distinctive living environment while maintaining the unique historical ambience of the community. Preservation and conservation issues dominate the underlying themes to be presented in the comprehensive planning program." 11

The Clarke County Business Park is currently the home to Berryville and the County's light industrial development. Most of the non-service major employers in Clarke County are located in the County Business Park. Among the top 10 private sector employer's 12 four - Berryville Graphics, American Woodmark, Caldwell & Santmyer and Cochran's Lumber and Millwork - are located on Jack Enders Boulevard.

The Clarke County Business Park is designated by the 2015 Berryville Area Plan to developed as Light Industrial or Research at a 0.3 Floor Area Ratio (FAR)¹³. The 2015 Plan does not represent a change, it has long been the intent of the County and Town to develop this area as light industrial or research.

The Berryville Area Plan Land Uses (2015) map is in Figure 1.5.

Clarke County and Berryville seek to maintain the rural setting and small-town nature of Berryville while providing new sources of employment and tax revenues. Expanding and controlling light industrial development to the Clarke County Business Park area will help achieve this goal

¹³ Ibid, page A-9 Table 12, Future Land Use Table and Projected Development Yields



¹¹ Berryville Area Development Authority, 2015 Berryville Area Plan, page III-1.

¹² Ibid, page I-5.

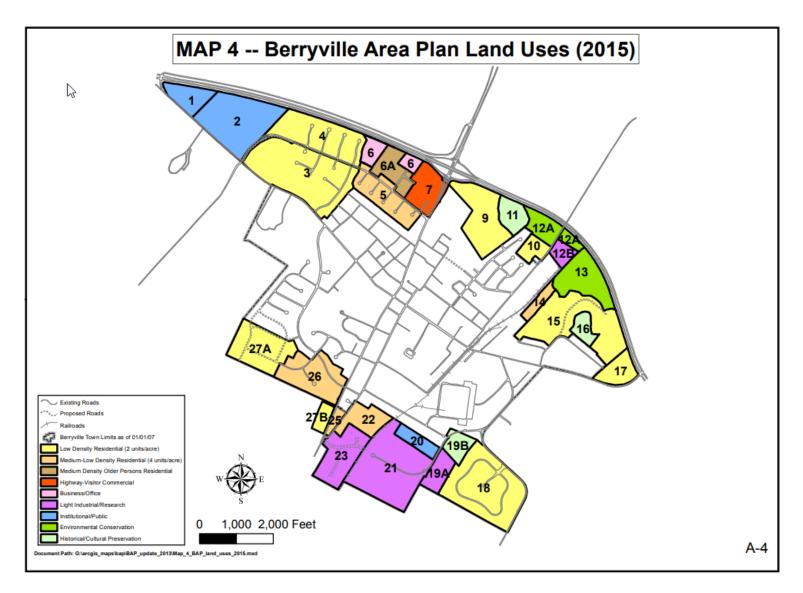


Figure 1.5 Berryville Area Plan Land Uses (2015) map. Source: 2015 Berryville Area Plan



2. Concept Alternatives

2.1 Concept Definition

Four Concepts for the Southeastern Collector were developed by Town and County Staff. These four Concepts do not inhibit the development of additional concepts, rather they define a general definition of all the possible alternatives for a Southeastern Collector roadway.

There are three general alternatives to extend Jack Enders Boulevard and expand the Business Park. A fourth alternative is a combination alternative. These alternatives, formally referred to as Concepts are below:

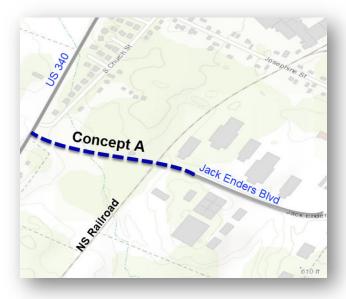
- 1. Concept A. Extend Jack Enders Boulevard over Norfolk Southern (NS) railroad to US 340.
- 2. Concept B. Extend Jack Enders to Smallwood Lane and improve Smallwood Lane to US 340.
- 3. Concept C. Extend Jack Enders Boulevard to US 340 and into Southern Potential Growth Area. This Concept is a combination of Concept A and D.
- 4. Concept D. New Road in Southern Potential Growth Area without a connection to Smallwood Lane.

Within each Concept there are several different alternatives. These alternatives were analyzed and those that provided distinct advantages and impacts were carried forward for further analysis. The result of this preliminary screening was one (1) Concept A, and C alternatives and two (2) Concept B and D alternatives.

Feasibility of New At-Grade Norfolk Southern Crossing

Although the Clarke County Business Park site plan shows an extension of Jack Enders Boulevard, the approval for a new-at-grade crossing is beyond the Town or County's control. As such, one of the objectives of this study is to determine the feasibility of a new crossing, and specifically of Concept A. The feasibility of a new at-grade crossing depends on the answer to two key questions:

- 1) What would be necessary to obtain approval from Norfolk Southern for a new crossing?
- 2) Is the Jack Enders Boulevard extension the most optimal location for a new collector in the southeastern growth area?





Multiple conversations were held with Norfolk Southern staff (see conversation summary below). New, at grade crossings are not only highly discouraged by Norfolk Southern, but also by Virginia State Statute¹⁴. Two hypothetical scenarios were discussed with Norfolk Southern Staff to gage the feasibility of a new crossing. First, what if the Town closed two or more existing at grade crossings? Second, would Norfolk Southern be amenable if the Town created a new siding and rail related commercial area.

The Norfolk Southern staff were <u>not</u> amenable to these hypothetical scenarios. Norfolk Southern Staff cited Virginia State Statues, and current efforts to eliminate at grade crossings.

Furthermore, the proposed location of the new at grade crossing for Concept A is located on a curve and has sight distance limitations for southbound trains. Although Norfolk Southern staff were careful not to formally reject the proposed at grade crossing, they were very clear that it was not a realistic a concept¹⁵.

Multiple Conversations with Norfolk Southern and Rail Personnel were conducted:

- On April 3, 2019 Mr. David Metcalf spoke with Scott Overbey, the Public Projects coordinator for Norfolk Southern (NS). The purpose of this conversation was to determine what the Town and County would need to do to gain approval from NS for a new at grade crossing. Mr. Overbey stated that the Town and County would need to follow the procedure in the Public Projects Manual. The Public Projects Manual calls for preliminary design, normally performed by NS at the community's expense.
- On May 16, 2019 Mr. David Metcalf spoke with Scott Overbey, the Public Projects coordinator for Norfolk Southern (NS). This conversation was a follow-up to the previous conversation to determine what could be done to obtain a decision from NS without submitting the engineering design and study. Mr. Overbey suggested that if the Town could develop a commercial rail terminal, then this development may justify an additional at grade crossing.
- On May 16, 2019 Mr. Metcalf spoke with Debra Haislip, State Rail Program Manager at Virginia
 Department of Transportation. Ms. Haislip stated that VDOT is working with NS and others to
 eliminate at-grade crossings, and that VDOT would aid with roadway project development if
 approved by NS.
- On May 23, 2019 Mr. David Metcalf spoke with Scott Overbey, the Public Projects coordinator for NS with the director for commercial development. The purpose of this conversation to determine if Berryville would be attractive to have rail related development. The NS personnel stated that a specific proposal would be necessary to determine if NS would provide rail infrastructure to support rail related development.
- On June 19, 2019 a teleconference was held with Mr. Roger Bennett (NS Corp) and Mr. Scott Overbey (NS Corp) and Christy Dunkle, Keith Dalton, David Ash, Brandon Stidham from Town and County. NS personnel highlighted additional issues with the proposed location of the new at grade crossing. Mr. Bennett suggested the town submit a Concept Package to NS for consideration.

¹⁵ Teleconference held June 19, 2019. Attendees from Norfolk Southern: Mr. Roger Bennett (NS Corp) and Mr. Scott Overbey (NS Corp).



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¹⁴ Va. Code Ann. § 56-363 (1996).

To obtain approval of a new at grade crossing, Norfolk Southern normally requires a Concept Package be submitted¹⁶. The review of this package, and subsequent plans are normally reviewed by Norfolk Southern at the requesters expense. It is likely that the Town would need to enter into a Preliminary Engineering agreement with Norfolk Southern and provide compensation to Norfolk Southern to continue applying the new at grade crossing.

The coordination with Norfolk Southern, research and analysis concluded:

- 1. The location of the new crossing for Concept A is not acceptable to Norfolk Southern. Even extraordinary efforts by the Town and County will be insufficient to overcome the general aversion to new at grade crossings, and especially to the proposed location.
- 2. The Town and County do not have existing crossings that can be eliminated to continue a dialog with Norfolk Southern. Nor does the Town and County have plans for major investment in a rail facility to make the new crossing part of a larger package.
- 3. To further pursue the new crossing for Concept A will require developing a Concept Package, and possibly enter into a Preliminary Engineering agreement with Norfolk Southern, and incur the expense of Norfolk Southern engineering reviews.
- 4. The location of Concept A is not superior to the other options. Concepts B and D create more developable property than Concept A. Concepts B and D have less wetland impacts than Concept A. Concept A's only advantage is lower construction cost.

As such, the Study Team recommended that the Town and County discontinue consideration of Concept A as the proposed Southeastern Collector. Concept C is a combination of Concept A and Concept D; as such this concept is also eliminated.

Final Concepts Selected for Study

Concept B and Concept D were selected for further study. Within both concepts there are two variations, resulting in four Concepts, shown on the following page in Figure 2.1:

- 1. Concept B1- Extend Jack Enders Boulevard into Smallwood Property, perpendicular to Craig's Run and onto Smallwood Lane. Upgrade Smallwood Lane and Smallwood Lane existing at grade crossing of Norfolk Southern RR.
- 2. Concept B2 Similar to B1; however, the roadway will run further east to take advantage of existing Smallwood Lane.
- 3. Concept D1 Identical to B1, without the connection to Smallwood Lane. This alternative will not provide a crossing of Norfolk Southern for the expanded Business Park.
- 4. Concept D2 Identical to B2, without the connection to Smallwood Lane. This alternative will not provide a crossing of Norfolk Southern for the expanded Business Park.

¹⁶ Norfolk Southern Railway Company, Public Projects Manual, Rev 2; Section 4 page 3.



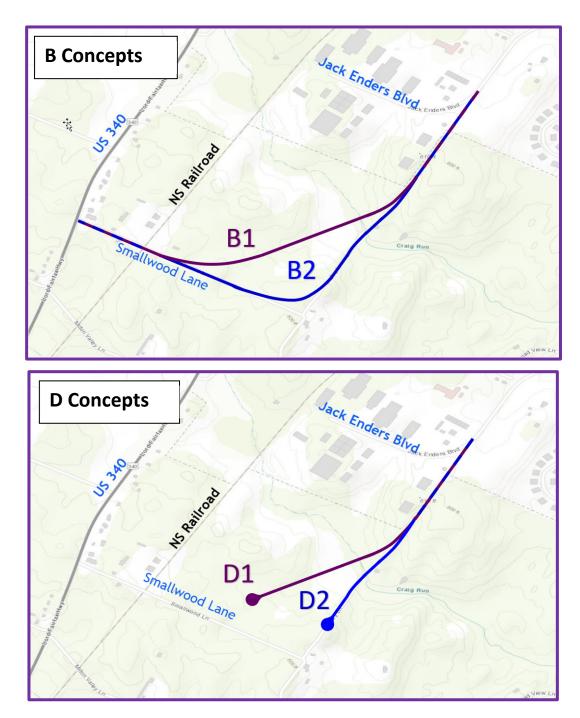


Figure 2.1: Concepts for Southeastern Collector



Characteristics of the Concepts

The proposed collector is planned as a rural collector. The design criteria for this roadway specifies 12 foot lanes, 4 foot paved shoulders and superelevation¹⁷. The design speed in rolling terrain is 50 mph.

Although the design speed does not specify the posted speed limit, it does lead to geometric criteria that accommodate that speed. A summary of criteria for the rural collector:

- Design speed (rolling terrain): 50 mph
- Minimum width of lane: 12 feet
- Minimum width of graded shoulders:
 8 feet; with 4' paved.
- Minimum radius: 760 feet.

Existing Jack Enders Boulevard (shown to right) generally meets these criteria, with the exception of paved shoulders. Although it is



desirable that the typical section of the existing Jack Enders Boulevard match the new section, it is not required. Furthermore, it is possible that a design exception could be obtained for the proposed roadway. However, for planning and cost estimating purposes, the criteria and typical section for fully compliant rural section are assumed.

Figure 2.2 shows the typical Section of the proposed collector. The assumed ROW is 70 feet with a 13-foot separation between the edge of pavement and the shared use path. At the RR crossing the shoulders are reduced and the ditches are eliminated, as such the required width is 32 feet.

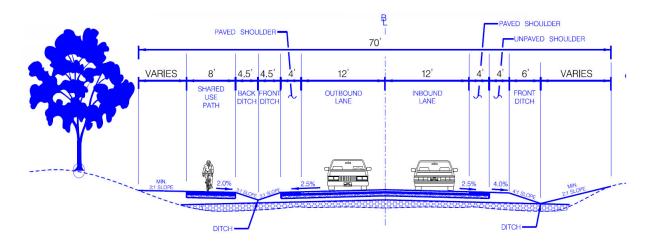


Figure 2.2: Typical Section of Southeastern Collector

¹⁷ Superelevation is banking of a roadway on curves and is not found on low volume or residential roadways.



The Concepts will extend Jack Enders Blvd and create a new intersection with the last portion of Jack Enders Blvd. This new intersection is the same for all of the Concepts and is shown in the extract below and in Figure 2.5 on the following page. Additional Concept Plans are in Appendix D.



Figure 2.3: Northern terminus of Concept D1

Concept B1 and Concept B2 will create an expanded intersection at US 340 and Smallwood Lane. This expanded intersection will require northbound deceleration and acceleration lanes. North of this intersection US 340 widens to a three-lane section. This three-lane section will be carried further south to allow for a dedicated southbound left turn lane onto Smallwood Lane. These improvements are shown in Figure 2.5 and in the extract below.

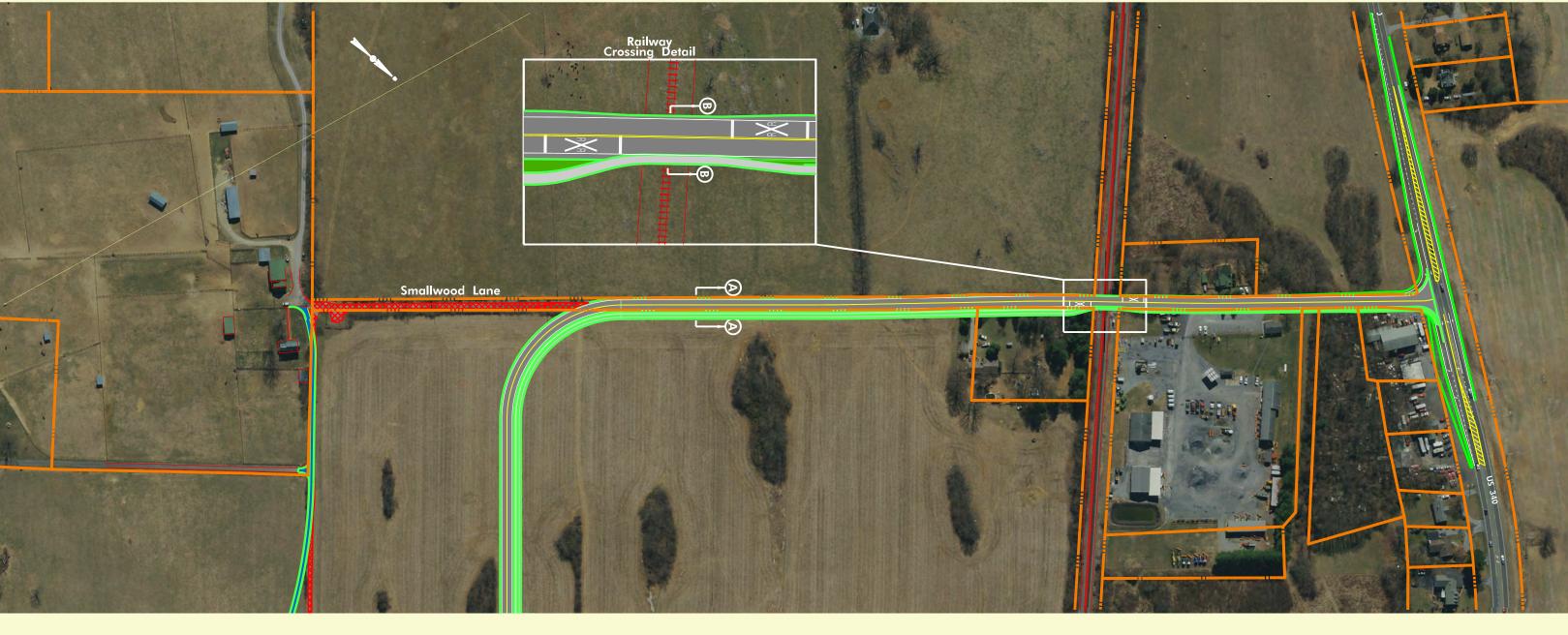
The expanded intersection at US 340 and Smallwood Lane will generally fit within the US 340 Right-of-Way (ROW). The ROW of the businesses just to the north of Smallwood Lane are currently using VDOT ROW for signs and access, some of this area will be used for the expanded intersection.



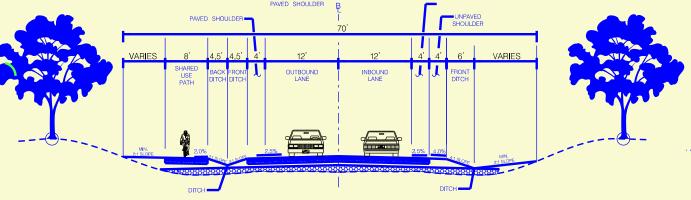
Figure 2.4: Southern terminus of Concept B1 and B2

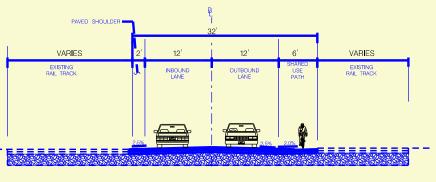


Berryville Roadway Improvements CONCEPT B1 - PART 1







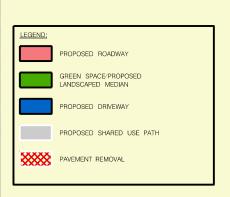


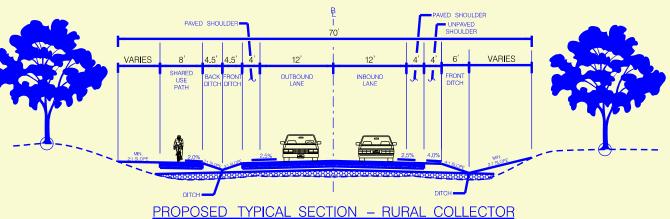
PROPOSED TYPICAL SECTION AA - RURAL COLLECTOR

PROPOSED TYPICAL SECTION BB - RURAL COLLECTOR

Berryville Roadway Improvements CONCEPT B1 - PART 2







3. Concept Analysis

3.1 Methodology

To determine which of the four concepts best meets the needs of the Town of Berryville and Clarke County, VA a quantitative and qualitative analysis was conducted. This analysis determined the relative costs, benefits and impacts of each concept and this section explains the results in the following subsections:

- Land Use the amount of property made available by the Concept and ability of each alternative to promote development.
- Traffic Service the ability of alternative to improve overall circulation, reduce trucks in downtown Berryville. Change in traffic volumes at key intersections and a general evaluation of new traffic patterns.
- Natural Environment Impacts to wetlands, forested areas, streams, flora and fauna and animals.
- Historical Resources Impacts to historic buildings, areas and sites. Federal, State and Local sites are considered.
- Community Impacts Impacts to neighborhoods and community facilities are estimated.
- Costs Construction and ROW costs are determined.

3.2 Land Use

This benefit is evaluated for:

- a) How much property is made available for development by the improvement?
- b) How likely is the development to occur relative to the other Concept alternatives?

The Clarke County Business Park is zoned BP Business Park and regulated under Section 612 of the Town of Berryville Zoning Ordinance. This designation allows for a maximum allowable density of 0.35 acres of developable land. This means that for every acre of developable land on the property, 0.35 acres can be built on. However, it is unlikely that every parcel will be developed to the maximum allowable density. A separate analysis was conducted to determine the future development.

The analysis assumed that future development has the same characteristics as the existing Business Park. Fourteen parcels of light industrial services were identified with an existing total rooftop area of about 301,000 Square Feet. The parcels cover a total land area of about 65 acres, with an estimated 44 acres of developable land. This translates to about 4,600 square feet of building per acre of developable land, or a ratio of 0.16 acre/acre.

The Business Park (BP) designation allows many types of uses including heavy industrial production such as Textile Product Mills and Heavy Construction. It also allows light industrial, which generates smaller consumer goods. The analysis assumes light industrial.

Concept Alternative B1, and B2. All the alternatives take advantage of the Smallwood Property between the existing Commercial Park and Smallwood Lane. The B1 alternative could also take



advantage of the properties to the south of Smallwood Lane, however that property – Milton Family Farm - is in an agricultural easement.

Alternative B2 is located east of B1 and could take advantage of the property to the east. This extension would likely be after the full buildout of the Smallwood Property.

An estimated 500,000 square feet are made available by each of the alternatives. The precise yield will depend on the development patterns and feeder road system. Figure 3.1 shows a scenario that will create 500,000 square feet of developable property.

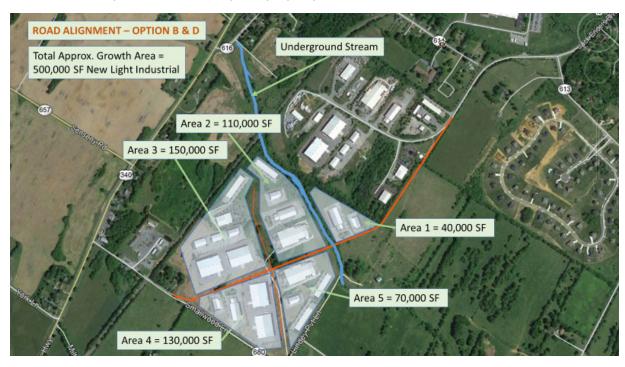


Figure 3.1: Theoretical Build-out of Smallwood Property.

Concept Alternatives D1 and D2. Similar to B1 and B2 the D Concepts can take advantage of the Smallwood property. As with the B Concepts, D2 is better positioned for a future buildout to the east. However, the improved accessibility of the new connection to US 340 will make the Smallwood Property more attractive. A summary of the analysis:

| Table 3.1 Summary of Land Use Benefits | | | | | |
|--|---|---|------------|------------|--|
| | Concept B1 | Concept B2 | Concept D1 | Concept D2 | |
| New SF Commercial / Light Industrial Land Use | 500k SF | 500k SF | 500k SF | 500k SF | |
| Future Expansion to East | Possible | Better | Possible | Better | |
| Attractiveness to Development | Better, Good connection to US 340 | Better, Good connection to US 340 | Possible | Possible | |



3.3 Traffic Service

3.3.1 Overview. Each Concept is assumed to bring light industrial development of approximately 500,000 Square Feet. The B1 and B2 Concepts provide new access to US 340. These concepts also provide a de facto bypass of Berryville along Jack Enders Boulevard extended. This additional development and the new connection will change traffic patterns in Berryville. The analysis was conducted to:

- Determine what improvements are needed, if any, at Jack Enders Boulevard and East Main Street for each Concept.
- Determine what upgrades to the intersection of US 340 and Smallwood Lane will be necessary with the B1 and B2 Concepts.
- Determine the general changes in traffic in downtown Berryville from each of the Concepts.

To answer these questions, the microsimulation AIMSUN was used to model the Town's road-network. Three intersections were analyzed with SNYCHRO to determine changes in Level of Service (LOS) along US 340 and Main Street. The three study intersections are:

- East Main Street and Jack Enders Boulevard,
- Main Street and US 340,
- US 340 and Church Street.

In addition, the intersection of US 340 and Smallwood Lane was analyzed for future conditions with Concept B1 and B2.

3.3.2 Traffic Methodology. Traffic projections in Berryville are challenging. There are many alternative routes for traffic to use – traffic can use Rt 7 to bypass Main Street or traffic can divert (i.e. "cut through") to local roadways to avoid downtown. Furthermore, traffic is dynamic. When an intersection becomes congested, traffic will divert away from that intersection until the intersection becomes less congested.

To meet these challenges and to show the changes in traffic flow an AIMSUN microsimulation of the Town was created.:

- The entire Town road network was downloaded and processed.
- Traffic was assigned using a 16 X 16 PM peak hour Origin Destination Matrix. The OD Matrix consisted of 5 external nodes and 11 internal Transportation Analysis Zones (TAZ).
- Trip generation from the 11 different internal TAZs were estimated from an inventory of buildings in the TAZ (Appendix A-4)
- Average Daily Traffic (ADT) from the VDOT database and Turning Movement Counts taken for the study at the three study intersections were used to calibrate and validate the model.

The process to develop the road network, to create the 16 X 16 Origin Destination Matrix and to validate the model are explained further in Appendix A: Traffic Analysis Methodology.

The analysis focuses on the three study intersections; however, general trends throughout the network as predicted by the model were observed as well.



3.3.3 Changes in Traffic Patterns. The analysis determined the following for Concept B and Concept D at the three study intersections. The figures shown are the changes in intersection volumes when compared with existing traffic.

Changes in traffic with Concept B at Jack Enders Boulevard and East Main Street:

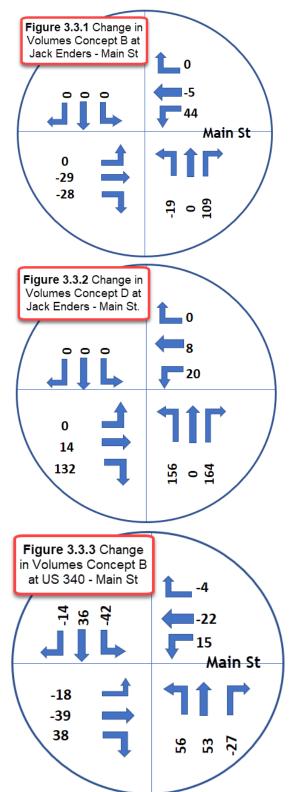
- The additional 500,000 square feet of light industrial was estimated to generate 450 new trips. Of these 340 new trips are *from* the development and 110 new trips *to* the development during the PM peak hour.
- ➤ Eastbound Traffic to Jack Enders Boulevard decreased, despite these new trips. This is due to traffic accessing Jack Enders Boulevard from the new connection from Smallwood Lane.
- The traffic impacts from the new development are largely mitigated with the new connection with US 340. The total volumes at the Jack Enders Boulevard / Main Street intersection increase slightly.

Changes in traffic with Concept D at Jack Enders Boulevard and East Main Street.

- ➤ Nearly all of the 450 peak hour trips will pass through this intersection.
- ➤ Without another access point, traffic on Jack Enders Boulevard more than doubles; from 301 vhp to 775 vph.
- As shown in Figure 3.3.2, the increase in left turns out of Jack Enders Boulevard increased by 156 vehicles per hour, and left turns onto Jack Enders Boulevard increased by 20 vph. These increases will warrant a new signal at this intersection.

Changes in traffic with Concept B at Main Street and US 340

- New traffic, to and from the south result in slight changes in traffic patterns through the intersection. Some of the decreases are due to traffic using Concept B to bypass the Town.
- > Overall, the intersection volumes increase by 67 vph.
- Some of the decreases (shown as negative numbers) are traffic diverted to smaller downtown streets.





Changes in traffic with Concept D at Main Street and US 340:

- ➤ The overall intersection volumes increase by 196 vph. Without traffic diverting to smaller downtown streets, this increase would be significantly greater.
- ➤ Nearly 200 vph are diverted to local roadways to avoid the US 340 — Main Street intersection. The mircrosimulation projected increases in the roadways shown in Table 3.2 on the following page.

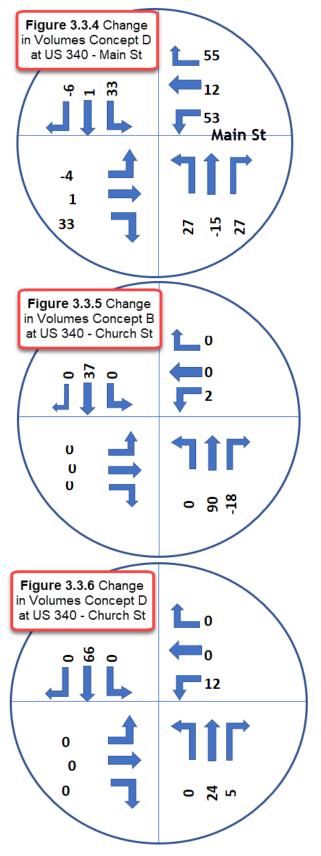
Changes in traffic with Concept B at US 340 and Church Street:

- ➤ The increase northbound of 90 vph is due to development traffic using the new connection to reach destinations to the north and west. This increase is also seen in the increase in left turns and through traffic at the Main Street intersection.
- The increase southbound of 37 vph is due to traffic traveling to the new development.

Changes in traffic with Concept D at US 340 and Church Street:

- Overall, traffic volumes are little changed from existing conditions.
- Additional traffic is traveling from the north or south, to or from the new development via US 340 and Main Street.

Projected traffic volumes are shown on the following page in Figure 3.4. The intersection of US 340 and Smallwood Lane, with Concept B has the projected traffic volumes shown in Figure 3.5.





US 340 / Church Street **Jack Enders Boulevard / Main Street** US 340 / Main Street **CONCEPT B** 397 3 436 48 2 205 **CONCEPT D** 362 78 370 71 2 260

Figure 3.4: Projected Traffic Volumes at Study Intersections.



3.3.4 Changes in Traffic Patterns. As a result of the improved connectivity, Concept B projects only minor changes throughout the Berryville roadway network. Table 3.2 shows that despite the addition of 450 new trips from the new development, the only significant increases are at the new, southern connection with US 340.

With Concept D, nearly 1/3 of the increased traffic volume are projected to use local streets to avoid the downtown area and the US 340 - Main Street Intersection. The microsimulation projects traffic to use the town grid, but most significantly North Church Street, Bundy Street, Academy Street, Liberty

Street and Page Street. Nearly 200 vehicles during the PM peak hour are projected to divert away from the intersection of US 340 and Main Street.

Concept D also exposes pedestrians and some residents to greater traffic. Along East Main Street there are residences, pedestrians and sidewalks. The additional business park traffic will expose pedestrians to more truck traffic. Closer to the center of Town the diverted "cut through" traffic will expose residents to external traffic.

The 450 vph new trips generated by the 500 thousand square feet of development results in increases in traffic throughout the town streets.



Figure 3.2: Concept D Diversion Routes Used (Shown in Green)

Table 3.3 shows the increases at the study intersections. The largest increases are at Jack Enders Boulevard and Main Street with Concept D and US 340 and Smallwood with Concept B.

| Table 3.2 Change in Traffic From Concepts | | | | | | | | | | | | |
|---|--------------|----------|----------|--|--|--|--|--|--|--|--|--|
| Location | Existing vph | Change B | Change D | | | | | | | | | |
| US 340 Main Street Intersection | 1381 | +32 | +216 | | | | | | | | | |
| Main Street / Jack Enders Intersection | 812 | +72 | +494 | | | | | | | | | |
| US 340 Church Street Intersection | 1092 | +111 | +107 | | | | | | | | | |
| US 340 – Smallwood Lane | 1040 | +407 | +62 | | | | | | | | | |

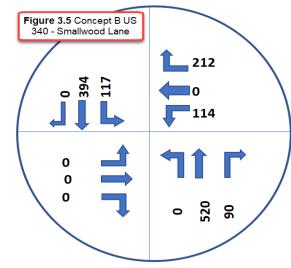
3.3.5 Concept B Bypass Traffic on Collector. Some traffic is projected to use the Concept B Collector to bypass downtown Berryville. For example, the model projects 50 vph from south to north to bypass the Town. However, the Origin Destination table assigns 60 vph to travel form points south on US 340 to points east on Rt 7. As such, most of the traffic making this movement is assigned by the model to use the new collector roadway. In addition, there are destinations along and north of Main Street that could also be used by the bypass. The overall potential is 45 vph and the model assigns 15 vph (1/3) of these trips to the Collector.

The model used current stop control at the intersection of US 340 and Jack Enders Boulevard, and some congestion did occur at this location. This congestion, in part, discouraged more traffic from using the Southeastern collector as a bypass. The potential for a larger volume using the collector as bypass is shown in Table 3.3.

| Table 3.3: Bypass Trips | | | | | | | | | |
|--|--|-----------------|--|--|--|--|--|--|--|
| From | То | Total Volume | Predicted to use the Collector as Bypass Route | | | | | | |
| Points South via US 340 | Points east via Main Street/ RT 7 | 60 vph | 50 vph | | | | | | |
| Points South via US 340 | NE quadrant of Town (1st Street and Battlefield Dr) | 45 vph | 15 vph | | | | | | |
| Points east via Main Street/ RT 7 | Points South via US 340 | 65 vph | 50 vph | | | | | | |
| NE quadrant of Town (1st Street and Battlefield Dr) | Points South via US 340 | 62 vph | 15 vph | | | | | | |
| Totals | | 232 vph | 130 vph | | | | | | |

3.3.6 US 340 - Smallwood Lane Intersection. Smallwood lane currently has very slight traffic, serving three residences, a business and the VDOT maintenance yard. With Concept B the combination of Smallwood Lane and Jack Enders Boulevard extended will serve a large area and provide a route to bypass downtown Berryville. This results in a new expanded intersection on US 340 and the likely need for a new signal at this intersection. The level of service of this intersection and the other study intersections is discussed in the next section.

Using tables from the Manual of Uniform Traffic Control Devices (MUTCD) the volumes at the US 340 and Smallwood Lane intersection do not justify a new traffic signal. At full



build; however, with heavy truck traffic, the highspeed through volumes, nearby railroad tracks and



the multipurpose trail along Concept B, a signal will provide an additional measure of control and safety to the intersection, and it may be required by VDOT when traffic volumes warrant it.

3.3.7 Concept B Smallwood Lane / Norfolk Southern Queue Analysis. The distance between Norfolk Southern Railroad tracks and US 340 is 770 feet. SYNCHRO was used to estimate the queues between US 340 and the Railroad tracks to confirm that traffic would not backup over the tracks (westbound traffic) or onto US 340 from the tracks (eastbound). For the eastbound queue, a five-minute wait for a passing train was assumed. The queue analysis showed enough roadway length, as shown in Table 3.4:

| Table 3.4: Queue Lengths Concept B | at US 340 and Sm | nallwood Lane with |
|---------------------------------------|------------------|--------------------|
| | EB with 5 Minute | WB with normal |
| | Stoppage | signal operations |
| Maximum Queue | 637 feet | 230 feet |
| 95% Queue | 483 feet | 125 feet |
| Average Queue | 128 feet | 84 feet |

- **3.3.8 Projected changes in Level of Service**. With only modest increases at most of the study intersections, the Level Of Service (LOS) of the three study intersections are little changed by the additional volume. The most significant change is at the intersection of Jack Enders and Main Street.
- ➤ **US 340-Main Street.** Only minor changes in LOS at this intersection was projected by the analysis. The LOS remains at LOS C for both Concept B and Concept D. Concept D has slightly more delay that Concept B.
 - The microsimulation model routed trips away from this intersection during the simulation as congestion became greater. There were slower, but relatively uncongested local and residential streets that became more attractive. With the advance of route-finding applications, such as Waze or Google Maps, these diversions are realistic.
- ➤ Jack Enders Boulevard/Main Street Improvements Needed. With Concept B, the intersection of Jack Enders Boulevard and Main Street will not need to be signalized. However, a new westbound left turn lane is needed and was assumed in the analysis. With Concept D a new left turn lane, and a new traffic signal will be required. Unsignalized analysis showed excessive queues for the northbound movement.
 - A signal is also assumed at US 340 and Smallwood Lane for Concept B; however this signal would not be necessary until full buildout of the Business Park.
- ➤ US 340 at Church Street. Volumes for both B and D increase on US 340 and the left turning vehicles from Church Street have difficulty gaining access to southbound US 340. However, there are several alternatives for traffic from Church Street to access US 340 South. With Hermitage Boulevard, Taylor Street or Swam Avenue also providing access to US 340 South, the left turn from Church Street is not expected to be problematic



| Table 3.5 Changes i | n Level of Service. | | | |
|-----------------------------|---------------------|--------------|------------------------|------------------------|
| | | Existing | Projected Concept B | Projected Concept D |
| US 340 – Main Stre | et | PM Delay Sec | PM Delay Sec | PM Delay Sec |
| | Northbound | 21.0/ C | 16.2/B | 24.8/C |
| | Southbound | 23.9/ C | 25.0/C | 27.6/C |
| | Eastbound | 28.5/ C | 25.5/C | 28.1/C |
| | Westbound | 27.8/ C | 29.2/C | 33.4/C |
| | | 25.0/C | 23.5/C | 28.8/C |
| Main Street – Jack | Enders Boulevard | | | |
| | Northbound | 13.8/ B | 12.9/B | 27.6/C |
| | Southbound | 12.6/ B | 14.8/B | 19.8/B |
| | Eastbound | 0.5/ A | 0.7/A | 32.0/C |
| | Westbound | 1.0/ A | 2.3/A | 26.5/C |
| | | | | 28.3/C |
| US 340 – Church Sti | reet | | | |
| | Northbound | (0.0) | (0.0) | (0.0) |
| | Southbound | (0.0) | (0.0) | (0.0) |
| | Westbound | 29.6/ D | 39.6/E | 40.0 / E |
| | | | | |
| US 340 – Smallwoo | d Lane | | | |
| | Northbound | NA | 4.1/A | NA |
| | Southbound | NA | 4.1/A | NA |
| | Westbound | NA | 9.5/A | NA |
| | | | 5.3/A | |
| Notes: Bold indicate | es unsignalized ana | alysis. | | |

3.3.9 Planning for Signalized Intersection Control

Intersections are converted from stop control to signalized control when safety, intersection delay or other operational condition justify the change. To assist traffic engineers with making this decision, the Manual for Uniform Traffic Control (MUTCD) has nine considerations, referred to as Warrants, that the traffic engineer analyzes. An example of Warrant 3 is on the following page.

If one or more of these warrants are met, the addition of a signal may be justified. If none of the warrants are met, the signal is normally not justified. A signal may be put into place without meeting one

MUTCD Traffic Signal Warrants

- 1. Warrant 1, Eight-Hour Vehicular Volume
- 2. Warrant 2, Four-Hour Vehicular Volume
- 3. Warrant 3, Peak Hour
- 4. Warrant 4, Pedestrian Volume
- 5. Warrant 5, School Crossing
- 6. Warrant 6, Coordinated Signal System
- 7. Warrant 7, Crash Experience
- 8. Warrant 8, Roadway Network
- 9. Warrant 9, Intersection Near a Grade Crossing

of the nine Warrants, however there would need to be extenuating or unusual circumstances. VDOT discourages the addition of signals for anything other than safety or operational issues.



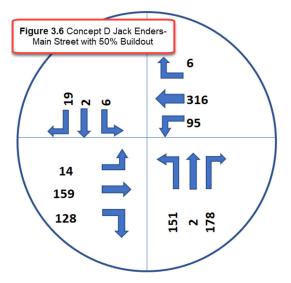
The VDOT will wait for traffic to build up to add a signal. Traffic projections may justify the new signal, but VDOT is not likely to place a signal based only on contingent projections. As such, at Jack Enders Boulevard and Main Street and at US 340 and Smallwood Lane, new signals will not be placed until the actual traffic justifies the signal. In such cases VDOT will require that the Southeastern Collector design incorporate the signal and include some of the signal infrastructure in the roadway project.

3.3.10 Phased Implementation of Development and Improvements. The proposed 500,000 square feet of new light industrial development is not likely to occur suddenly. It may not even be light

industrial; Section 4: Implementation, discusses a need for the Town to seek all types of desirable development. The build up to 450 new peak hour trips, will take time to achieve.

The negative traffic impacts of Concept D are apparent after full implementation. A key question is at what point will these negative impacts become apparent. To answer these questions, 50% of full buildout was evaluated. Figure 3.6 shows Jack Enders Boulevard – Main Street Volumes with 50% of full buildout.

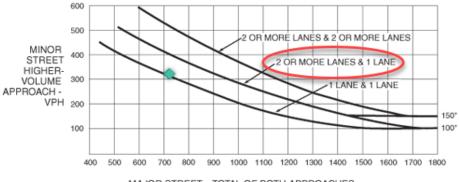
With improvements to Jack Enders Boulevard a signal is not yet warranted. Figure 3.7 shows Warrant 3, Peak Hour. The green dot shows that under this warrant a signal does not meet the warrant with an additional turn lane on Main Street. Turn lanes would normally be added to the major street (Main Street) before installation of a new signal.



Concept D at 50% of full buildout:

- Need for Signal No;
- Need for Left Turn WB Lane Yes;
- Need for Right Turn EB Lane Yes;
- Diverted Traffic in Berryville approximately ½ of Full Buildout 100 vph.





MAJOR STREET—TOTAL OF BOTH APPROACHES— VEHICLES PER HOUR (VPH)

*Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.



Concept B1 could be constructed in four phases. First, an extension from Jack Enders Boulevard into the Smallwood property would be built to support development. Next, as development progresses, improvements to the Jack Enders Boulevard / Main Street Intersection would be built. As development approaches 75% of buildout, the Connector will need to extend to US 340. Finally, a signal at the intersection of Smallwood Lane and US 340 will be necessary. This progression is shown in Figure 3.8.



Figure 3.8: Timing of Connector Construction

The timing of the improvements in Figure 3.8 will depend on the type of development and corresponding number of trips generated. When development reaches 25% of full buildout approximately 125 thousand SF of development will have occurred. For a typical industrial park this will create an additional 870 new daily trips and 107 new peak hour trips. This typical scenario is not likely to trigger the need for roadway improvements, however if the new development has greater than typical trip generation or a high number of trucks then new roadway improvements may be necessary.

Traffic Analysis Summary

Findings

- ➤ The development associated with the Collector roadway is projected to be 500,000 Square Feet (SF) of light industrial. This development is projected to generate 340 vph *from* the area and 110 vph *to* the area (PM peak hour).
- The downtown Berryville Main Street Intersections are projected to have modest increases in traffic volume and intersection delay.
- Additional traffic from Concept D is expected to use local streets to avoid the center of Berryville. With Concept D nearly 200 vehicles during the PM peak hour are projected to divert away from the intersection of US 340 and Main Street.
- Concept B creates a route to bypass downtown Berryville. The model projects 130 vph to use Concept B as a bypass under normal conditions (PM peak hour).



- > The traffic at the Jack Enders Boulevard / Main Street intersection will increase by 494 vph, over 60%, with Concept D. Current total volume is 812 vph.
- With Concept D additional capacity is needed at Jack Enders Boulevard and Main Street. A new signalized intersection with a dedicated westbound left turn lane and eastbound right turn lane will be necessary.
- Initially the negative impacts from Concept D will be manageable. A new signal at Jack Enders Boulevard will not be necessary until development is well underway.
- > Traffic on Smallwood Lane will increase from 10 vph to 533 vph with Concept B. This will create the need for improvements to US 340 and, eventually, a new signal.

Traffic Conclusions. The improved connectivity provided by Concept B provides traffic benefits in Berryville. Without the additional connection development related traffic will divert to local streets and the Jack Enders Boulevard / Main Street intersection will require significant improvements.

3.4 Natural Environment

All the concepts are largely located in farmland, as such impacts to habitat, forests, and endangered species are estimated to be minimal. The impacts will be limited to Craig's Run, an intermittent stream which is surrounded by wetlands, and a freshwater forested wetland. Craig's Run is listed on the EPA 303d list for impaired waters in Virginia for E-coli from NPS agricultural runoff. The wetlands fall under section 404 of the Clean Water Act requiring delineation and permitting for any impacts.

The B1 and D1 Concepts have the least impacts to wetlands. They were developed to have a cleaner, more direct perpendicular crossing of Craig's Run that minimizes the area impacted. Nonetheless the B2 and D2 Concepts are still estimated to impact only 1 acre of wetlands.

Although none of the Concepts will have a major effect on Craig's Run, the impacts to wetlands and forestlands will need to be delineated and a Joint Permit Application to the Virginia Department of Environmental Quality will need to be conducted.

Impacts to Forestland is also minor. The B2 and D2 Concepts have less, but comparable, impacts to forests. A summary of impacts to wetlands and forest lands are in Table 4.2.

According to the Department of the Interior, several potential threatened/endangered species may exist in the study area including: Myotis sodalis (Indiana bat), Myotis septentrionalis (Northern Longeared bat), and Antrolana lira (Crustacean or unpigmented troglobite). There may also be a Bartramia longicauda, (Upland sandpiper) in the region though no sightings have been identified since 2014. Although none of these species' habitat is expected to be adversely affected, a survey of the area will be necessary prior to design and construction of the new collector roadway.

The following protocol will need to be followed for all options.

- A Joint Permit application to the Department of Environmental Quality to determine jurisdiction of DEQ alone or The United States Army Corp of Engineers.
- Impacts to wetland and forestlands will need to be determined by delineations and analysis, ideally, this will assist in avoiding mitigation for any displacement



- Threatened and Endangered species review will be required by US Fish and Wildlife Service (Section 7 review)
- Karst geology considerations for surface limestone, underground seeps/springs and sinkholes.
- There will be additional DEQ permits for stormwater management and erosion and sediment control standards.
- A scenic buffer may be required by the conditions of the easement along Smallwood Lane.

| Table 3.6 Summary of Natural Environment Impacts | | | | | | | | | |
|--|------------|------------|-------------|------------|--|--|--|--|--|
| Concept B1 Concept B2 Concept D1 Concep | | | | | | | | | |
| Impacts to wetlands | .50* acres | 1.0 acres | 0.50* acres | 1.0 acres | | | | | |
| Impacts to Forests | 1.79 acres | 1.64 acres | 1.79 acres | 1.64 acres | | | | | |

^{*}Impacts could be minimized depending upon strategy for construction implemented.

3.5 Historical Resources

Although the Berryville area is rich in historic structures, the Concepts only impact a cluster of buildings located along Lindey Lane. The Concepts will not have direct or indirect impacts to the Josephine City Historic District or the Milton Valley Cemetery.

The structures identified along Lindey Lane were listed on the Clarke County inventory of potentially historic facilities. They have not been protected through the Commonwealth of Virginia or the National Trust for Historic Preservation. It is unclear from discussion with the County or local historic professionals the historic value of these structures. Both Concept B2 and D2 will be near these structures. Further investigation may be required if options will displace them.

| Table 3.7 Summary of Historic Resource Impacts | | | | | | | | | | |
|--|---|------------------------|------|------------------------|--|--|--|--|--|--|
| | Concept B1 Concept B2 Concept D1 Concept D2 | | | | | | | | | |
| Historic Structures within 100 feet | none | 3 County Designated | none | 3 County Designated | | | | | | |

3.6 Community Impacts

The impacts of the one-mile collector roadway through the undeveloped area are slight. Most of the community impacts are as a result of increases in traffic volumes and are identified in Section 3.3.

The most significant changes to Berryville and the surrounding area are due to the change in land use. The development of approximately 500,000 square feet of light industrial will change the appearance of the area just south and east of Berryville. However, as with the existing Business Park, the area can be easily secluded from rest of the town. The casual visitor to Berryville is probably unaware that the existing business Park, with 550,000 square feet of light industrial exists.



To further understand potential impacts to the Berryville area, an outreach program was conducted with key stakeholders. Berryville Graphics and representatives from Top of Virginia Chamber were interviewed. Full meeting notes are in Appendix A, a summary of each interview:

Berryville Graphics – Christy Dunkle and David Metcalf met with Gary Rannells and other Berryville Graphics Staff. The Berryville Graphics publishing operation takes deliveries and makes shipments generally to the west on Route 7. They have approximately 550 employees spread in 2 or 3 shifts, and they generally come from the north and west.

Mr. Rannells and other staff did not believe a new collector would hinder operations. There is the possibility that a new collector connection with US 340 will reduce the number of deliveries that mistakenly use Josephine Street.

Top of Virginia Regional Chamber. David Metcalf met with Cynthia Schneider of Top of Virginia Regional Chamber. Ms. Schneider did not place expansion of the business park as a high priority to support business in Berryville. However, if the new collector and expanded business park do not distract from the small-town charm of Berryville, then the existing visitor and tourist base would not be affected. She also believed that the limited bypass function the collector would perform would provide improvements to the existing network required and create community obligations.

There is ROW that needs to be purchased the entire length of each concept. These takings are not expected to impact the operations of any of the agricultural or businesses along the roadway. The ROW required by parcel is shown in Table 3.4 and in Appendix C: Construction and ROW costs.

State and Local governments in Virginia have the ability, via eminent domain, to acquire property from private landowners for transportation projects. The private landowner is entitled to fair compensation of their property plus damages. The Virginia Department of Transportation has a well-established process that involves an appraisal, negotiation and additional compensation for hardships created for the property owner.

Most of the private property needed is in the Smallwood property. At either terminus additional ROW is needed to provide for the 70-foot-wide typical section of the roadway. Property along the front or back of 14 parcels will be needed.

To the south of Smallwood Lane, the Milton Valley Farm is in a Forest-Agricultural Easement and is restricted from eminent domain takings. The ROW along Smallwood Lane is estimated to be 33 feet, as such the property needed to the north of Smallwood Lane is as much as 37 feet.

Concept B at the southern terminus requires an upgrade to the intersection. This upgrade will change the access to the auto sales / auto parts businesses to the north of Smallwood Lane. These businesses will have considerably more pass by-traffic and, if desired will be able to redevelop the properties.

The Collector with its associated development will add jobs, revenue and economic benefits to the area. It is not expected to have a negative impact on the Berryville community.



| | Table 3.8 Right o | f Way Requ | ired by Parc | el (Square Fe | et) |
|-------|---------------------------------|---------------|---------------|---------------|---------------|
| Parel | Description | Concept B1 | Concept B2 | Concept D1 | Concept D2 |
| I | NE corner US 340- Smallwood | 2,883 | 2,883 | | |
| II | SE US 340 - Smallwood Lane | 7,557 | 7,557 | | |
| III | North side of Smallwood Lane | 2,103 | 2,103 | | |
| IV | South side of Smallwood Lane | 7,404 | 7,404 | | |
| V | North side of Smallwood Lane | 894 | 894 | | |
| VI | North side of Smallwood Lane | 9,660 | 9,660 | | |
| VII | Residence just east of RR | 11,770 | 11,770 | | |
| VIII | Milton Valley Farm Property | 0 | 0 | | |
| IX | Smallwood Property | 231,511 | 249,372 | 188,121 | 149,848 |
| X | Pumpernickel Press | 14,818 | 6,245 | 14,818 | 6,245 |
| ΧI | Timberlake Cabinet | 8,576 | 8,576 | 8,576 | 8,576 |
| XII | Water tower | 2,402 | 2,402 | 2,402 | 2,402 |
| XIII | Along Jack Enders | 10,998 | 10,998 | 10,998 | 10,998 |
| XIV | West Side, Jack Enders | 18,468 | 18,468 | 18,468 | 18,468 |
| ΧV | West Side, Jack Enders | 0 | 0 | 0 | 0 |
| XVI | West Side, Jack Enders | 1,030 | 1,030 | 1,030 | 1,030 |
| XVII | West Side, Jack Enders | 3,803 | 3,803 | 3,803 | 3,803 |



3.7 Costs

Construction costs for the Concepts is estimated based on quantities of major cost elements and a contingency of 35%. The costs per mile are consistent with recent highway projects.

Intersection improvements and new signals were estimated for Jack Enders Boulevard and Main Street (Concept D only) and US 340 and Smallwood Lane (Concept B only). Improvements at the Smallwood Lane at-grade rail crossing was assumed to be performed by Norfolk Southern Railroad, but not funded by Norfolk Southern Railroad. The estimated cost is for similar, active crossing examples.

ROW was estimated based on \$10,000 / acre for Open Space (Smallwood Property) and \$20,000 / acre for residential or business property. The additional ROW along Smallwood Lane was reduced at the Norfolk Southern Rail crossing. No major utility relocations are anticipated in the project.

| Table 3.9 Comparative Costs (\$ thousands) | | | | | | | | | | |
|--|---------|---------|----------|----------|--|--|--|--|--|--|
| | B1 | B2 | D1 | D2 | | | | | | |
| Construction Cost | | | | | | | | | | |
| Mainline | \$8,130 | \$7,950 | \$5,280 | \$4,820 | | | | | | |
| Intersection Improvements | \$560 | \$560 | \$430 | \$430 | | | | | | |
| New Traffic Signals | \$600 | \$600 | \$540 | \$540 | | | | | | |
| At Grade Railroad Crossing | \$160 | \$160 | \$0 | \$0 | | | | | | |
| Subtotal Construction Cost | \$9,460 | \$9,280 | \$6,250 | \$5,790 | | | | | | |
| Total Length (feet) | 5,500 | 5,650 | 3,200 | 3,000 | | | | | | |
| Cost per mile | \$9,080 | \$8,670 | \$10,310 | \$10,200 | | | | | | |
| ROW Costs | \$100 | \$100 | \$60 | \$70 | | | | | | |
| Total Cost | \$9,560 | \$9,380 | \$6,310 | \$5,870 | | | | | | |

Concept D is shorter and consequently less expense than Concept B. The difference between B1 and D2 is slightly more than \$3.7 Million. Additional detail on the cost estimates are in Appendix C: Construction and ROW Cost Estimates.

3.8 Alternatives Analysis: Summary and Conclusions

Four Concepts were studied and developed. These Concepts are:

- Concept A. Extend Jack Enders Boulevard over Norfolk Southern (NS) railroad to US 340.
- 2. Concept B. Extend Jack Enders Boulevard to Smallwood Lane and improve Smallwood Lane to US 340.
- 3. Concept C. Extend Jack Enders Boulevard to US 340 and into Southern Potential Growth Area. This Concept is a combination of Concept A and D.
- 4. Concept D. New Road in Southern Potential Growth Area without a connection to Smallwood Lane.

Concept A and Concept C require a new at grade crossing of the Norfolk Southern Railroad. This new crossing was determined to be not feasible based on coordination with Norfolk Southern and Town of Berryville and Clarke County Staff. The extensive measures and mitigation that would need to be taken to provide minor benefits made these Concepts not feasible.

Within Concept B and Concept D two variations were developed. Concept B1 and Concept D1 cross perpendicular to Craig's Run and are located near the center of the Smallwood property. Concept B2 and D2 use the existing Lindey Lane. The advantages and disadvantages of each concept is as follows:

Concept B1 – This Concept is the most costly but provides the most benefit. The alignment splits the Smallwood Property providing a central roadway for the new business park. The additional connection to US 340 aids the flow of traffic from the new and existing business park.

The alignment also minimizes the impact to Craig's Run and surrounding wetlands. However, it is the costliest Concept with an estimated cost of \$9.6 Million

Concept B2 - This Concept is a variation of B1 and uses existing Lindey Lane. Compared with B1 it is located on the edge of the Smallwood Property, and has a less direct crossing of Craig's Run and increased environmental impacts. However, this Concept has the same traffic benefits as B1 and a slightly lower cost of \$9.4 Million.

Concept D1 – This Concept follows the B1 alignment, however it does not provide a new connection to US 340. As such it results in additional traffic through downtown Berryville and will require an upgrade to the intersection of Jack Enders Boulevard and East Main Street. The cost is lower than either of the "B" Concepts at \$6.3 Million.

Concept D2 – As with B2 this Concept is on the edge of the Smallwood Property and will not be a central roadway for the new business park. It also has a less direct crossing of Craig's Run, which increases the environmental impacts to Craig's Run. However, this Concept has the lowest cost of \$5.9 Million.

Both Concept D1 and D2 impact traffic flow in Berryville. Without the connection to US 340 vehicles will increasingly use local roadways to avoid downtown Berryville.

Table 3.10 summarizes the benefits and impacts:



| Table 3.10 Summary of Costs and Benefits | | | | | | | | | |
|--|--|----------------------------------|---|---|--|--|--|--|--|
| | B1 | B2 | D1 | D2 | | | | | |
| Land Use | Smallwood Lane | | Bisects Smallwood Lane Property | Eastern Edge of Smallwood Lane Property not optimal | | | | | |
| Environmental | Minimal impact to Craig's Run Not as environmentally preferred crossing of Craig's Run | | Minimal impact to Craig's Run | Not as environmentally preferred crossing of Craig's Run | | | | | |
| Traffic Flow | Improves traffic flow throughout | Improves traffic flow throughout | Large increase on Jack Enders Boulevard, traffic diversions onto Berryville streets | Large increase on Jack Enders Boulevard, traffic diversions onto Berryville streets | | | | | |
| Implementation | More \$\$, includes new connection at US 340. More \$\$, includes new connection at US 340. | | Can be expanded after initial phase. | Can be expanded after initial phase | | | | | |
| Total Cost | \$9,560 | \$9,380 | \$6,310 | \$5,870 | | | | | |

Concept B1 and B2 provide a connection to US 340 in the south which improves traffic flow for the business park and for traffic in the downtown area of Berryville. However, with the additional length B1 and B2 are more costly.

The new connection to US 340 is needed as the new business park gains momentum towards full build out. However, in the early phases of development the new connection to US 340 is not as necessary. Either Concept D1 or D2 will serve as an initial phase for Concept B1 or B2 with manageable impacts until the development is at least 50% complete.

The business park is likely to be on the Smallwood Property, and the Concept B1 / D1 best serves this property. It has the added benefit of minimizing impacts to Craig's Run and the properties along Lindey Lane.

Although Concept B1 best meets the needs of the Town and County, the roadway may be built in two phases. The first phase will be Concept D1. Once the area begins to attract business and light industry, the remainder of Concept B1 can be completed.



4.Implementation Plan

Funding Strategies

New Revenue from the business park can be expected to generate enough funds to capitalize the Southeastern Collector. However, timing is an issue. Without development plans it is difficult to attain grants, attract investors or gain proffers to design and build the roadway.

New public roadways in Virginia are generally funded by State and Federal sources via the Smart Scale prioritization process and the Revenue Sharing program. There are also a number of grants that are for related uses that could be applied towards the Southeastern Collector. This section identifies the steps the Town and County must take to define development, to be competitive with Smart Scale, and to obtain grants.

Smart Scale

Smart Scale¹⁸ prioritizes transportation projects to use a wide range of State and Federal funds. The program ranks projects based on objective and quantifiable criteria. Jurisdictions throughout the Commonwealth submit projects that generally compete within their Virginia Department of Transportation (VDOT) District. There are six criteria, and the weighting of the criteria vary depending on the location of the project. For example, in more urban areas congestion mitigation score is weighted 45%; in rural areas this score is weighted 10%.

Berryville and Clarke County compete in the VDOT Staunton District using a rural weighting of the scores. This score is then divided by the cost of the project to determine the final score. The cost is the actual amount of funding needed. The weight of scores in each category for rural (Category D¹⁹):

<u>Safety</u> 30%
<u>C</u>ongestion Mitigation 10%
<u>A</u>ccessibility 15%
<u>L</u>and Use Not used
<u>E</u>conomic Development 35%
Environmental Quality 10%

To be successful Berryville and Clarke County need to score well in either (or both) Safety and Economic Development. If the cost of the project is funded in part by another source, this will improve the score.

In the FY 2018 round, projects in Staunton District with scores over 2.18 were approved for funding²⁰. However, in the FY 2020 Staunton District the lowest score approved is 4.31²¹. Both the scoring and

²¹ Smartscale.org: FY 2020 Selected projects final (approved June 19, 2019).



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¹⁸ Commonwealth Transportation Board, SMART SCALE Technical Guide, February 21, 2018.

¹⁹ Ibid, page 36.

²⁰ Smartscale.org; Consensus scenario approved. May 17, 2017.

the approvals are relative, as such it is not possible to precisely predict points from round to round. However, what is necessary for the Southeastern Collector to be competitive can be determined from previous rounds and the scoring process.

Safety. The safety score is based in the estimated reduction in fatal and injury crashes. It is calculated for both the reduction in number of crashes and the reduction in crash rate. Berryville has a low number of crashes and a low crash rate; in the three years of 2016 to 2018 only 7 injury crashes were reported. As such, previous submissions have not been able to gain much of a score in this area.

Crashes are not entered into the State data base unless a police crash report is filed. It is possible that pedestrian or bike injuries are not reported, rather the injured person is taken immediately to seek treatment without the police crash report. In addition, the low number of crashes may also be a statistical abnormality and the number of crashes in 2019 may be closer to a true norm. An update of the Town and County crash record may help with the next Smart Scale submission.

The estimated decrease in crashes as a result of the project is estimated using Crash Modification Factors (CMF). CMFs vary by type of improvement. As such, the score may also be improved if the project includes proven safety features with large CMFs.

Economic Development. This score is one in which the Southeastern Collector can score well. Determining the Economic Development Score is complex and consists of three separate evaluations: Project Support for Economic Development (60%), Intermodal Access and Efficiency (20%) and Travel Time Reliability (20%).

for Economic Project Support Development rewards developments that are well along in site development. The table below shows factors used to scale the development square footage. For example, if the County / Town has the project referenced in Comprehensive Plan (it does), and a conceptual site plan approved for 220k SF, it would be given credit for 220k X 1.8 = 396k SF. Based on previous submissions, this amount of area would contribute 2.1 points to the total

| Rating Description | Point Value |
|--|----------------|
| Transportation project referenced in local Comprehensive Plan | 0.5 |
| Transportation project located in an area of economic distress* Up to: | 0.5 (0.3) |
| Development project site plan status: | |
| Conceptual site plan submitted: | 0.5 |
| Conceptual site plan approved: | 1.0 |
| Detailed site plan submitted: | 2.0 |
| Detailed site plan approved: | 4.0 |
| Total Maximum | 5.0 |
| *Berryville has distress score of 59.6/200 = 0.3 | |

Example Project Support for Economic Development Score Calculation:

- 1. Scaling Score: 1.8 (0.5 for Comp Plan, 0.3 for economic distress, 1.0 for approved plan)
- 2. Concept Site Plan Approved for 220k SF; 1.8 X 200k = 360k
- 3. Expected normalized value = 3.96,
- 4. 60% of Economic Development Score and 35% of total Score: .83,
- 5. Divide by cost in \$10 Million: .83 / \$.4 = 2.1points

Figure 4.1: Smart SCALE Economic Development Scoring and Example



(See Figure 4.1). This score will be higher if the conceptual plan is for more than 220k SF, and lower if more than \$4 million in funding is needed.

Intermodal Access and Efficiency can provide points if the development includes freight terminals. Travel Time Reliability, as currently calculated, will not provide additional points.

The next Smartscale round will occur in 2021 for the FY 2022 round. Considerable project development does need to occur to support the Southeastern Collector application. First and foremost, the town of Berryville will need to work with area agencies to adapt the local planning documents. These include: Land use — comprehensive plan. Town/County annotation plans showed comprehensive plans and additions should the plans grow. The following items require engineering or considerable staff time to prepare, much of which can be performed well in advance.

- Conceptual Plans
- Town and County Annexation Agreement
- Shared Comprehensive Plan
- o Traffic Counts
- Cost Estimate
- Governing Body Resolution of support
- MPO Resolution of support
- Site Development Plan(s)
- o Smart SCALE Application Portal

Smart SCALE Summary: The majority of points will come from the Safety and Economic Development categories. Safety will contribute points if additional injury crashes are recorded. Economic Development will provide points if there is an approved conceptual site plan. The level of effort for a Smart SCALE application is considerable, the Town and County should begin the longer lead time items well in advance.



Revenue Sharing

Revenue Sharing is a VDOT managed program that provides 50% funding from the State and requires a 50% local match. Projects apply on a biennial basis. New construction, reconstruction, and improvement projects are all eligible.

Projects need to be reviewed by the local VDOT Project Manager to confirm the eligibility of the project and to determine that the scope and estimate are accurate. This is done through the SMART Portal. Once this initial approval is provided, the detailed application can be submitted during the biennial application period.

After review by the VDOT Project Manager and Local Assistance Program personnel, and subject to the availability of funds, the project will receive a permanent UPC and will be submitted to the Commonwealth Transportation Board (CTB) for final approval.

The Southeastern Collector Roadway would need to be one of the Concept B alternatives to qualify for Revenue Sharing. As such, Town and County would need to obtain over \$4.3 million in local funds or roadway related grants to provide the local match.

Revenue Sharing: New Roadways

Revenue Sharing Program funds may be used to establish a new facility to be part of the system of state highways or part of the road system in the locality that is eligible to receive maintenance payments from VDOT pursuant to §33.2-319 of the Code of Virginia. In order for a new roadway to be eligible for Revenue Sharing Program funding, it must be a part of a locally adopted plan such as the locality's Comprehensive Plan and must be expected to divert sufficient traffic from existing public roads so that those roads will not need to be improved in the foreseeable future. Projects may also need to be included in the regional Constrained Long Range Plan in air quality non-attainment areas. Qualifying projects should provide an immediate benefit to the overall transportation network with a connection between two existing major public roads, based on current transportation needs. Projects that exclusively serve private developments or commercial establishments are not eligible. (Source: VDOT, Revenue Sharing Program Guidelines, page 4).



Grant Funding – Developing the Vision

Although most transportation funding sources are allocated through Smart SCALE there are several related State and Federal sources that are not included in Smart SCALE that can be used to reduce the funding needed. However, to compete for these sources there needs to be a clear vision and strategy for the site.

The future development potential for the campus can be accomplished with regional input through public planning meetings and focused visioning sessions. This visioning should consider:

- O What is a missing need for the region that this development could provide?
- O What sort of development will enhance the community?
- O What jobs are needed or desired?
- o The impact to schools or other community assets?

Once the vision for the site is determined, the vision will need to be incorporated into the Comprehensive Plan and other regional plans. After inclusion in the plans funding can be applied for from a variety of sources (see table 4.1). The strongest grant applications will be for projects that provide benefits on a regional scale, economic promise, technology sector improvements, or features to enhance community livability.

In addition to grants that provide benefits on regional scale, there are sources that are focused on individual aspects of the development. Funding sources to protect wetland, waterway and forests would add funds for the site, and indirectly to the Southeastern Collector. Complimentary options may also include the addition of smart grid designs for the campus which would enhance funding opportunities through the Redismart program. This program is evolving through the Department of Energy.

Infrastructure program funding is available through the INFRA program, for transportation projects that promote economic vitality, innovative technology and accountability. Smaller projects associated with connections to the National Highway Freight Network, are the priority for this program.

All of these funding opportunities require the campus and road development to be in the comprehensive plan with a vision for the final development goals. The development of site as flexspace, hotel/conference center, medical center, community college or education complex, or as a hub business facility will determine the most appropriate course to follow to apply for grant funding.

Private Development

Often private developers who own a property will pay for roadways and traffic signals to support their proposed development. For the Southern Potential Future Growth area and the Southeastern Collector, a private developer may be persuaded to purchase the Smallwood property and build the collector provided the Comprehensive Plan, proper zoning and the roadway concept are complete.

In addition, Virginia's Public Private Partnership (P3) program allows the local government to request proposals and select the proposal that best delivers the vision. This tool is often used when the government wishes to privatize their property to benefit the community. This option is most realistic



for the development of the Smallwood property if the Town or County can gain ownership or a contract with the current owner.

| Table 4.1 Potential Funding Sources | |
|---------------------------------------|--|
| State Grant Funder | Brief description |
| Community Development Block Grant | Based upon demographics and community need |
| FEMA flood protection policies and | Flood education, policy enforcement, construction standard |
| regional planning | updates, ordinance review |
| VDOT SRTS | Safe routes to schools, walking trails, bike trails |
| Go Virginia, Growth and Opportunity | Tech sector partnerships to develop economy in rural areas |
| Federal Grant Program | |
| USDA/NRCS Watershed Protection grants | For water quality, water supply protection, habitat |
| US Forest Service Land and Water | Way to purchase land for permanent protection |
| Conservation Fund | |
| TIGER/Build grants | Public transportation program 20% for urban areas |
| Redismart, department of Energy | For smart grid design implementation |
| INFRA program | Transportation that promotes economic vitality, |
| | accountability along freight highway |

Summary - A Way Forward

This Study establishes the Southeastern Collector's traffic benefits, impacts and costs. This Study also provides the conceptual engineering needed for the Smart Scale and Revenue Sharing programs. However, to complete the Smart Scale application, a vision for the Southern Potential Future Growth area needs to be established. This vision will need to be shown in the approved Town and County Comprehensive plans and eventually a site plan will need to be developed.

The conceptual plan of the Southeastern Collector and the Comprehensive Plans will allow for additional grants to be obtained. Complimenting the development with other community needs such as clean water projects, forest development, technology innovations and walking/bike trails will allow for additional funding opportunities to match the major funding needed for the roadway.

Private developers can, at any time, hasten the process if they can be persuaded to implement the vision. This may be accomplished through a Public Private Partnership (P3). A large, comprehensive P3 will allow a single investor taking over the Smallwood Property and the construction of the roadway. The feasibility of this option is dependent on the marketability of the vision. If not feasible for a large P3, then a more conventional approach will be to build a portion of the roadway, and then induce development of individual parcels.

Phased construction of the Collector with incremental development will allow developers and the State and Federal sources to be used. In this regard, Concept D1 or D2 will serve as an initial phase for Concept B1 or B2.

A suggestion implementation plan:



Initial Planning, Smart SCALE Preparation

- 1. Develop a vision for the Southern Potential Future Growth area and update Comprehensive Plans.
- 2. Determine marketability of site development visions. Identify tenant types, and progress site plans.
- 3. Obtain Governing Bodies Resolution of support.
- 4. Submit for FY 2022 Smart Scale funding.

Obtain Grants and Support from Developers

- 5. Determine infrastructure requirements in addition to the Southeastern Collector.
- 6. Identify and apply for grants and other funding to support infrastructure requirements.
- 7. Determine feasibility of P3 to take over infrastructure and development. Develop and execute P3 arrangement if feasible.
- 8. Identify and market tenants and developers (if not P3).

Design, Gain Permits and Build

- 9. Design and gain permits for Southeastern Collector and other infrastructure.
- 10. Construct initial phases of Southeastern Collector via grants and initial developer(s).
- 11. Complete Southeastern Collector when the new business park is a legitimate growing enterprise.

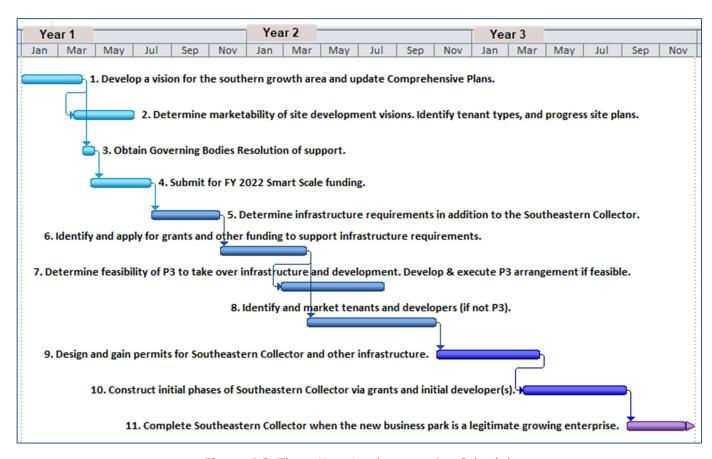


Figure 4.2: Three Year Implementation Schedule



5. Conclusions and Recommendations

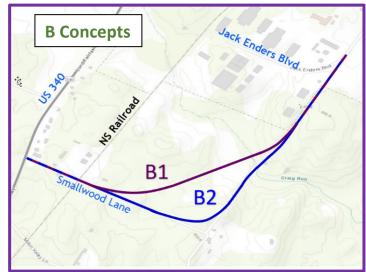
After reviewing a wide range of alternatives, Concept B1 was determined to best meet the needs of the Town and County. The Concept provides an upgraded crossing of the Norfolk Southern Railroad, improved traffic flow in and around Berryville and best promotes future development in the Southern

Potential Future Growth Area. The estimated cost is \$9.6 Million.

A new at-grade crossing of the Norfolk Southern Railroad was considered and discussed at length with Norfolk Southern staff. This new crossing is simply not feasible and this Concept is deleted from further consideration.

Planning, funding and building the new roadway will require a multi-pronged and multi-phased approach.

First, the Southern Potential Future Growth Area, primarily on the Smallwood property,



will need to be planned. With a vision of how this property will be redeveloped the Town and County can add this vision to the Comprehensive Plan and update the zoning. These initial planning steps will allow the roadway to compete for Smart SCALE funding and open the potential for grants and investment from private developers.

Second, the Town and County need to actively pursue funds from State and Federal Grants and investment from developers. An extension of Jack Enders Boulevard into the Smallwood Property, similar to Concept D1, will encourage some initial development on the Property. With this initial development it will be easier to attract other users or developers to the property. The site will be able to generate revenue and provide the funds to finish the Collector with either Smart SCALE or Revenue Sharing.

With a marketable vision the site and roadway will be a candidate for a Virginia Public Private Partnership (P3). This program will allow the Town and County to contract the development of the site. A private entity will assume much of the funding and risk, and in turn receive either future revenues or profits from the site.

Smart SCALE is the dominant program for allocating State and Federal transportation funds. The Southern Potential Future Growth Area will need to be planned for the Southeastern Collector to gain Economic Development points. Without Smart SCALE, the Town and County can also receive a 50% match using the Revenue Sharing program. With Concept B1, the Town and County will need to raise \$4.8 Million (one half of \$9.6 M). Other grants and private money can be used for this match.



In the previous Section, a three-year schedule is suggested. Depending on the vision and plan for the site, there are many ways the roadway and the accompanying development can proceed. Three years is optimistic as there are three years of worth of activities that need to take place.

Concept B1 could be implemented in four phases:

- V. Extend Jack Enders Boulevard into the Smallwood property to stimulate initial development.
- VI. As development progresses, improve the Jack Enders Boulevard / Main Street Intersection.
- VII. As development approaches 75% of buildout, extend the Connector to US 340.
- VIII. When traffic warrants, add a signal at the intersection of Smallwood Lane and US 340.

The timing of the improvements will depend on the type of development and corresponding number of trips generated. When development reaches 25% of full buildout approximately 125 thousand SF of development will have occurred. For a typical industrial park this will create an additional 870 new daily trips and 107 new peak hour trips. This typical scenario is not likely to trigger the need for roadway improvements, however if the new development has greater than typical trip generation or a high number of trucks then new roadway improvements may be necessary.

When the development reaches 50% of buildout and 250 thousand SF of development improvements to Jack Enders Boulevard will be necessary. At this point it will also be necessary to gain environmental approvals and begin design of the Connector. By 75% of buildout and 375 thousand SF of development it will be time to complete the Connector to US 340.

The final improvement is a signal at US 340 and Smallwood Lane. This improvement should be implemented when conditions warrant, likely after 75% development.

The Southeastern Collector and the accompanying development will create many benefits for the Town and County. We recommend that the Town and County select Concept B1 and begin the visioning and planning for the associated development.





Appendix A:

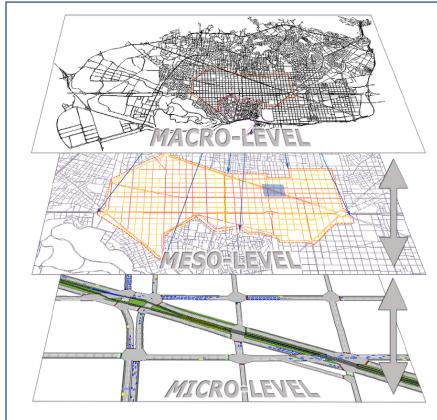
Traffic Analysis Methodology

APPENDIX A: Traffic Analysis Methodology

The heart of the Southeastern Collector Transportation Study are the traffic projections and analysis of these projections. The projections and analysis are needed to determine the benefits and impacts of each of the Concepts. They need to determine:

- How much traffic will use each Concept, and how much external traffic will be diverted to each Concept?
- How will traffic patterns change, especially in downtown Berryville?
- How much traffic will use Concept B as a bypass of Berryville?
- What traffic control is necessary at new or expanded intersections:
 - Jack Enders Blvd and Main Street;
 - US 340 and Smallwood Lane;
 - US 340 and Concept A.
- What are the before and after LOS for the study intersections?

Projecting traffic in the Town of Berryville is challenging. There are many alternative routes for traffic to use – traffic can use Rt 7 to bypass Main Street or traffic can divert (ie "cut through") to local roadways to avoid downtown. Furthermore, traffic is dynamic. When an intersection becomes congested, traffic will divert away from that intersection until the intersection becomes less congested.



Three levels of Traffic Models:

Macro Level provide projections over a large area on principal roadways. Metropolitan Planning Organizations normally maintain Macro models.

Meso models are similar but in more detail and less coarse than the Macro Model.

Microsimulations seek to duplicate existing conditions. Each vehicle in a microsimulation is an independent program. Vehicles can be observed as they traverse the network.

Source of image: AIMSUN.com



As a result of the many route choices and dynamic nature of Berryville traffic, a dynamic traffic model was selected to model the traffic. A fully calibrated and validated microsimulation with dynamic route assignment of the Town would best perform this analysis. Such a microsimulation creates a detailed digital duplicate of the Town, however it a very time consuming and expensive approach.

Meso traffic models have many of the same benefits as the microsimulation, however they do not graphically show the flow of vehicles in the same manner as the microsimulation. A microsimulation that is at the same detail as the Meso Traffic Model will have the benefits of both. AIMSUN was selected to perform this analysis. AIMSUN can more easily provide a microsimulation model of the town and with modest calibration and validation efforts do so at the same accuracy as a Meso traffic model.

Highway Capacity Manual (HCM) Level of Service (LOS) analysis is the standard to determine how well a roadway will function. The LOS levels A through F are familiar and well accepted. To determine the LOS of the key intersections SYNCHRO was selected for its ease of use, ability to modify signal timing and phasing and ability to duplicate HCM analysis. The SYNCHRO results are in Appendix B: Traffic Counts and LOS Worksheets.

Traffic counts were taken at three intersections. In addition, VDOT AADT from the 2018 Daily Traffic Volume Estimates, Special Locality Report 168 Berryville was used to determine conditions at other locations in the network. All analysis was based on PM peak hour.

The following steps were taken to produce the AIMSUN model:

- Develop the road-network. A full AMISUN model of Berryville was downloaded and processed. Links were updated, unnecessary links were eliminated and characteristics of links were modified to duplicate existing conditions.
- 2. **Create Transportation Analysis Zones**. Five external zones and 11 internal zones were created.
- 3. **Develop Origin (OD) Destination Matrix**. The Five external OD locations and 11 internal areas were developed to create a 16 X 16 OD Matrix. This matrix identifies were traffic originates from (origin) and goes to (destination).
 - a. For each external zone the existing traffic counts informed how many vehicles originate from the zone and how many pass into and through the model.
 - b. For each internal zone the number and type of buildings and playing fields were used to estimate how many vehicles originate from the zone or leave the zone.
 - c. Row and Column totals were calculated from a and b above. Local knowledge, engineering judgement were used to populate the remainder of the OD matrix. Iterative steps were required to balance the matrix.
- **4.** Calibrate and Validate Existing traffic. The AIMSUN model was run with Dynamic Route Assignment (DRA). The model was adjusted to reflect realistic conditions and to approximate traffic counts at the key intersections. This is a very time consuming process and the required an understanding of how vehicles are likely to flow through and around Berryville.
- **5. Determine Traffic Pattern Changes with Concepts.** The AIMSUN model was run with the new development traffic (TAZ 16) and new links. The change in traffic volumes was noted.

Improvements to the network were not assumed for the analysis. A background growth rate was not assumed as well. To clearly answer the key questions it was determined that the existing network, with the existing traffic as the base traffic would be best. Additional assumptions as to network improvements and background growth adds another variable to an already complex analysis.



On this and the following pages:

Figure A.1 – AIMSUN Network

Figure A.2 – Location of External and Internal TAZ

Figure A.3 – 16 X 16 PM Peak Hour OD Matrix

Figure A-1: AIMSUN Network



Figure A-2: Location of External and Internal TAZ

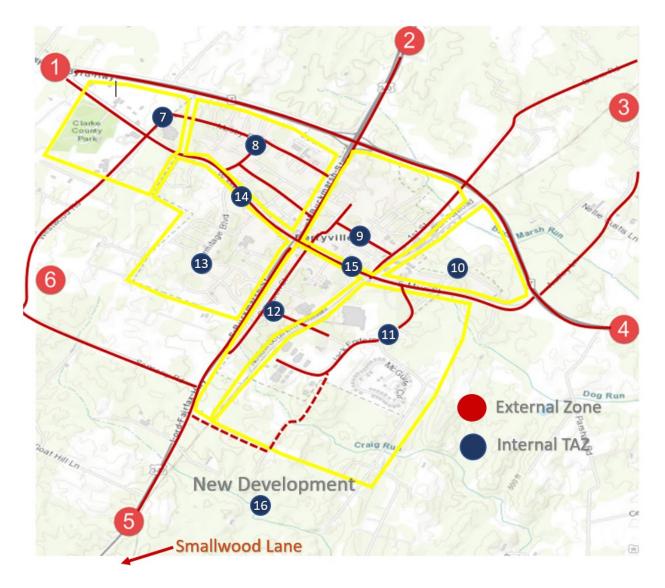


Figure A-3: 16 X 16 PM Peak Hour OD Matrix

| | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 | 13 | 14 | 15 | 16 | Total |
|-------|------|------|----|------|-----|-----|------------|-----|-----|------------|-----|-----|-----|-----|----|----|-------|
| 1 | | 360 | 10 | 1890 | 55 | 20 | 15 | 70 | 20 | 15 | 40 | 10 | 15 | 35 | 25 | 15 | 2600 |
| 2 | 360 | | 5 | 250 | 100 | 5 | 15 | 25 | 40 | 5 | 35 | 5 | 5 | 30 | 20 | 12 | 1440 |
| 3 | 5 | 5 | | 10 | 5 | 2 | 5 | 5 | 5 | 2 | 5 | 2 | 2 | 15 | 5 | 3 | 50 |
| 4 | 1840 | 250 | 10 | | 65 | 10 | 10 | 70 | 30 | 5 | 45 | 10 | 15 | 35 | 20 | 15 | 2300 |
| 5 | 75 | 120 | 10 | 60 | | 80 | 15 | 40 | 15 | 10 | 35 | 66 | 10 | 30 | 15 | 13 | 576 |
| 6 | 15 | 20 | 10 | 15 | 30 | | 10 | 5 | 15 | 5 | 5 | 5 | 5 | 25 | 10 | 3 | 100 |
| 7 | 30 | 25 | 5 | 10 | 15 | 15 | | 25 | 30 | 10 | 5 | 10 | 20 | 8 | 5 | 3 | 190 |
| 8 | 60 | 20 | 5 | 60 | 10 | 5 | 22 | | 10 | 5 | 5 | 5 | 5 | 12 | 7 | 3 | 220 |
| 9 | 80 | 100 | 15 | 60 | 40 | 25 | 25 | 20 | | 10 | 5 | 15 | 20 | 20 | 20 | 3 | 515 |
| 10 | 25 | 20 | 3 | 7 | 7 | 3 | 3 | 3 | 3 | | 3 | 5 | 5 | 20 | 10 | 2 | 85 |
| 11 | 40 | 45 | 5 | 70 | 35 | 10 | 5 | 10 | 5 | 5 | | 5 | 5 | 10 | 10 | 4 | 280 |
| 12 | 35 | 10 | 3 | 5 | 30 | 5 | 15 | 3 | 3 | 3 | 3 | | 3 | 10 | 5 | 3 | 115 |
| 13 | 40 | 10 | 3 | 5 | 30 | 5 | 1 5 | 5 | 10 | 3 | 5 | 4 | | 10 | 5 | 4 | 130 |
| 14 | 35 | 30 | 15 | 35 | 30 | 25 | 8 | 12 | 20 | 20 | 10 | 10 | 10 | | 20 | 5 | 220 |
| 15 | 35 | 20 | 5 | 20 | 15 | 10 | 5 | 7 | 20 | 1 0 | 10 | 5 | 5 | 20 | | 5 | 100 |
| 16 | 50 | 55 | 5 | 80 | 45 | 15 | 6 | 11 | 8 | 8 | 8 | 8 | 8 | 15 | 15 | | 340 |
| Total | 2600 | 1760 | 50 | 2300 | 384 | 100 | 140 | 300 | 365 | 60 | 260 | 150 | 205 | 140 | 80 | 90 | |



Appendix B: Traffic Counts and Level of

Service Worksheets

Contents:

- 1. Traffic Counts
- 2. SYNCHRO Level of Service Worksheets

Phone: 703 914-4850

File Name: Main Street @ Jack Enders Blvd

Site Code : 0006 Start Date : 5/21/2019

Page No : 1

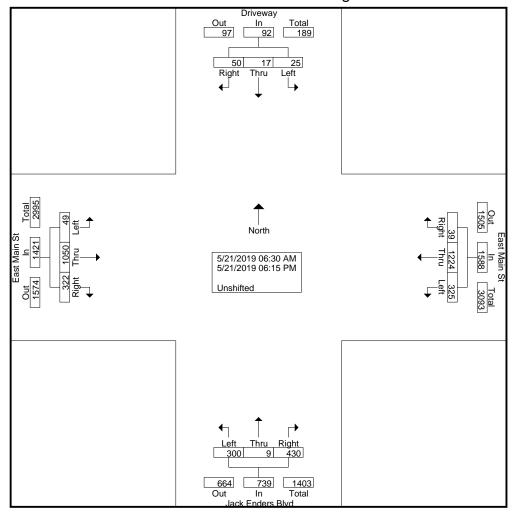
Groups Printed- Unshifted

| | | Drive |)W2V | | | Eact N | lain St | Group | S FIIIILE | | ders Bl | ıd | | Eact N | /lain St | |] | | |
|-------------|------|-------|-------|---|------|--------|---------|---------|-----------|------|---------|-----|-------|--------|----------|---|--------------|--------------|------------|
| | | From | | | | From | | | Jo | From | | vu | | | West | | | | |
| Ctout Times | Left | | | | Left | | | | 1.04 | | | | l oft | | | | | | 1.4 7.4-1 |
| Start Time | | Thru | Right | | | Thru | Right | U Turns | Left | Thru | Right | | Left | Thru | | | Exclu. Total | Inclu. Total | Int. Total |
| 06:30 AM | 0 | 0 | 0 | 0 | 28 | 18 | 0 | 0 | 2 | 0 | 15 | 0 | 0 | 55 | 25 | 0 | 0 | 143 | 143 |
| 06:45 AM | 0 | 0 | 1 | 0 | 34 | 17 | 0 | 0 | 4 | 0 | 11 | 0 | 3 | 70 | 45 | 1 | 1 | 185 | 186 |
| Total | 0 | 0 | 1 | 0 | 62 | 35 | 0 | 0 | 6 | 0 | 26 | 0 | 3 | 125 | 70 | 1 | 1 | 328 | 329 |
| | | _ | _ | _ | | | _ | _ | | _ | | | _ | | | _ | | | |
| 07:00 AM | 1 | 0 | 0 | 0 | 20 | 28 | 0 | 0 | 15 | 0 | 34 | 0 | 0 | 43 | 27 | 0 | 0 | 168 | 168 |
| 07:15 AM | 0 | 0 | 1 | 0 | 29 | 17 | 1 | 0 | 10 | 0 | 17 | 0 | 2 | 66 | 28 | 0 | 0 | 171 | 171 |
| 07:30 AM | 0 | 2 | 1 | 0 | 12 | 25 | 2 | 0 | 27 | 0 | 47 | 1 | 3 | 65 | 12 | 0 | 1 | 196 | 197 |
| 07:45 AM | 3 | 1 | 3 | 0 | 24 | 36 | 4 | 0 | 18 | 1 | 17 | 0 | 4 | 74 | 11 | 0 | 0 | 196 | <u>196</u> |
| Total | 4 | 3 | 5 | 0 | 85 | 106 | 7 | 0 | 70 | 1 | 115 | 1 | 9 | 248 | 78 | 0 | 1 | 731 | 732 |
| 00 00 444 | | | | _ | | 40 | | • | | | 4- | | _ | 70 | 4.0 | • | | 400 | 400 |
| 08:00 AM | 1 | 1 | 2 | 0 | 17 | 18 | 1 | 0 | 14 | 1 | 17 | 0 | 2 | 78 | 16 | 0 | 0 | 168 | 168 |
| 08:15 AM | 1 | 2 | 3 | 0 | 8 | 17 | 3 | 0 | 3 | 0 | 20 | 0 | 3 | 53 | 12 | 0 | 0 | 125 | 125 |
| 08:30 AM | 0 | 1 | 2 | 0 | 6 | 27 | 0 | 0 | 9 | 1 | 19 | 0 | 1 | 53 | 13 | 0 | 0 | 132 | 132 |
| 08:45 AM | 0 | 1 | 2 | 0 | 4 | 29 | 0 | 0 | 8 | 0 | 10 | 0 | 2 | 39 | 10 | 0 | 0 | 105 | 105 |
| Total | 2 | 5 | 9 | 0 | 35 | 91 | 4 | 0 | 34 | 2 | 66 | 0 | 8 | 223 | 51 | 0 | 0 | 530 | 530 |
| ! | _ | | _ | _ | | | | _ | | _ | | . 1 | _ | | | _ | | | |
| 09:00 AM | 0 | 1 | 0 | 0 | 11 | 26 | 1 | 0 | 5 | 0 | 13 | 1 | 0 | 33 | 10 | 0 | 1 | 100 | 101 |
| 09:15 AM | 0 | 1 | 2 | 0 | 7 | 24 | 1 | 0 | 5 | 0 | 11 | 1 | 2 | 36 | 5 | 0 | 1 | 94 | 95 |
| | | | | | | | | | | | | | | | | | _ | | |
| Total | 0 | 2 | 2 | 0 | 18 | 50 | 2 | 0 | 10 | 0 | 24 | 2 | 2 | 69 | 15 | 0 | 2 | 194 | 196 |
| | | | | | | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | | | |
| 1 | _ | _ | _ | _ | | | | _ | | | | - 1 | | | | _ | | | |
| 03:30 PM | 2 | 0 | 6 | 0 | 8 | 66 | 1 | 0 | 39 | 1 | 54 | 0 | 4 | 42 | 20 | 0 | 0 | 243 | 243 |
| 03:45 PM | 1_ | 1_ | 3 | 0 | 9 | 79 | 3_ | 0 | 11_ | 1_ | 16 | 0 | 3 | 37 | 8 | 0 | 0 | 172 | 172 |
| Total | 3 | 1 | 9 | 0 | 17 | 145 | 4 | 0 | 50 | 2 | 70 | 0 | 7 | 79 | 28 | 0 | 0 | 415 | 415 |
| 1 | | | | | ı | | | | ı | | | 1 | | | | | 1 | | |
| 04:00 PM | 2 | 0 | 8 | 0 | 19 | 77 | 2 | 0 | 11 | 0 | 17 | 0 | 4 | 37 | 8 | 0 | 0 | 185 | 185 |
| 04:15 PM | 1 | 1 | 2 | 0 | 11 | 90 | 0 | 0 | 12 | 0 | 9 | 0 | 3 | 36 | 10 | 0 | 0 | 175 | 175 |
| 04:30 PM | 3 | 0 | 4 | 0 | 14 | 74 | 4 | 0 | 22 | 0 | 21 | 0 | 3 | 42 | 5 | 0 | 0 | 192 | 192 |
| 04:45 PM | 2 | 1_ | 0 | 0 | 22 | 80 | 2 | 0 | 16 | 1 | 17 | 0 | 1 | 29 | 14 | 0 | 0 | 185 | 185 |
| Total | 8 | 2 | 14 | 0 | 66 | 321 | 8 | 0 | 61 | 1 | 64 | 0 | 11 | 144 | 37 | 0 | 0 | 737 | 737 |
| | | | | | ı | | | | ı | | | | | | | | | | |
| 05:00 PM | 4 | 2 | 1 | 0 | 7 | 96 | 1 | 0 | 23 | 0 | 31 | 0 | 3 | 31 | 7 | 0 | 0 | 206 | 206 |
| 05:15 PM | 0 | 0 | 1 | 0 | 9 | 76 | 0 | 0 | 11 | 0 | 12 | 0 | 1 | 29 | 11 | 0 | 0 | 150 | 150 |
| 05:30 PM | 1 | 0 | 2 | 0 | 6 | 82 | 1 | 0 | 13 | 2 | 7 | 0 | 2 | 23 | 9 | 0 | 0 | 148 | 148 |
| 05:45 PM | 1 | 1 | 4 | 0 | 12 | 96 | 3 | 0 | 8 | 1 | 4 | 0 | 1_ | 28 | 2 | 0 | 0 | 161 | 161 |
| Total | 6 | 3 | 8 | 0 | 34 | 350 | 5 | 0 | 55 | 3 | 54 | 0 | 7 | 111 | 29 | 0 | 0 | 665 | 665 |
| | | | | | | | | | · | | | | | | | | | | |
| 06:00 PM | 1 | 1 | 2 | 0 | 8 | 64 | 0 | 0 | 7 | 0 | 7 | 0 | 1 | 25 | 7 | 0 | 0 | 123 | 123 |
| 06:15 PM | 1 | 0 | 0 | 0 | 0 | 62 | 9 | 0 | 7 | 0 | 4 | 0 | 1 | 26 | 7 | 0 | 0 | 117 | 117 |
| Grand Total | 25 | 17 | 50 | 0 | 325 | 1224 | 39 | 0 | 300 | 9 | 430 | 3 | 49 | 1050 | 322 | 1 | 4 | 3840 | 3844 |
| Apprch % | 27.2 | 18.5 | 54.3 | | 20.5 | 77.1 | 2.5 | | 40.6 | 1.2 | 58.2 | | 3.4 | 73.9 | 22.7 | | | | |
| Total % | 0.7 | 0.4 | 1.3 | | 8.5 | 31.9 | 1 | | 7.8 | 0.2 | 11.2 | | 1.3 | 27.3 | 8.4 | | 0.1 | 99.9 | |
| | | | | | | | | | | | | · | | | | | | | |

Phone: 703 914-4850

File Name: Main Street @ Jack Enders Blvd

Site Code : 0006 Start Date : 5/21/2019

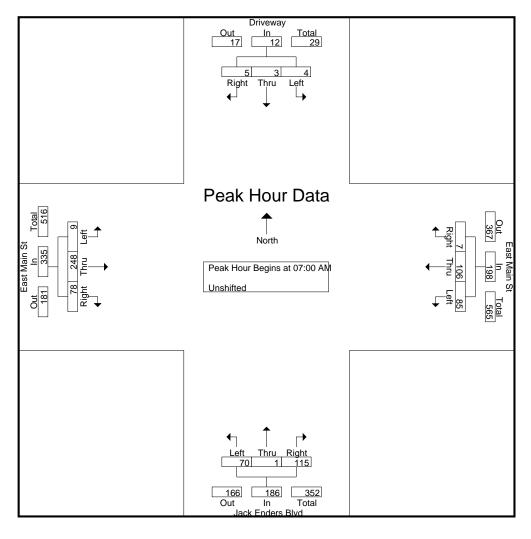


Phone: 703 914-4850

File Name: Main Street @ Jack Enders Blvd

Site Code : 0006 Start Date : 5/21/2019

| | | | eway | | | | Main St | | | | ders Bl | /d | | | | | |
|--|------|------|-------|------------|------|------|---------|------------|------|------|---------|------------|------|------|-------|------------|------------|
| | | From | North | | | Fron | n East | | | From | South | | | | | | |
| Start Time | Left | Thru | Right | App. Total | Left | Thru | Right | App. Total | Left | Thru | Right | App. Total | Left | Thru | Right | App. Total | Int. Total |
| Peak Hour Analysis From 06:30 AM to 09:15 AM - Peak 1 of 1 | | | | | | | | | | | | | | | | | |
| Peak Hour for Entire Intersection Begins at 07:00 AM | | | | | | | | | | | | | | | | | |
| 07:00 AM | 1 | 0 | 0 | 1 | 20 | 28 | 0 | 48 | 15 | 0 | 34 | 49 | 0 | 43 | 27 | 70 | 168 |
| 07:15 AM | 0 | 0 | 1 | 1 | 29 | 17 | 1 | 47 | 10 | 0 | 17 | 27 | 2 | 66 | 28 | 96 | 171 |
| 07:30 AM | 0 | 2 | 1 | 3 | 12 | 25 | 2 | 39 | 27 | 0 | 47 | 74 | 3 | 65 | 12 | 80 | 196 |
| 07:45 AM | 3 | 1 | 3 | 7 | 24 | 36 | 4 | 64 | 18 | 1 | 17 | 36 | 4 | 74 | 11 | 89 | 196 |
| Total Volume | 4 | 3 | 5 | 12 | 85 | 106 | 7 | 198 | 70 | 1 | 115 | 186 | 9 | 248 | 78 | 335 | 731 |
| % App. Total | 33.3 | 25 | 41.7 | | 42.9 | 53.5 | 3.5 | | 37.6 | 0.5 | 61.8 | | 2.7 | 74 | 23.3 | | |
| PHF | .333 | .375 | .417 | .429 | .733 | .736 | .438 | .773 | .648 | .250 | .612 | .628 | .563 | .838 | .696 | .872 | .932 |

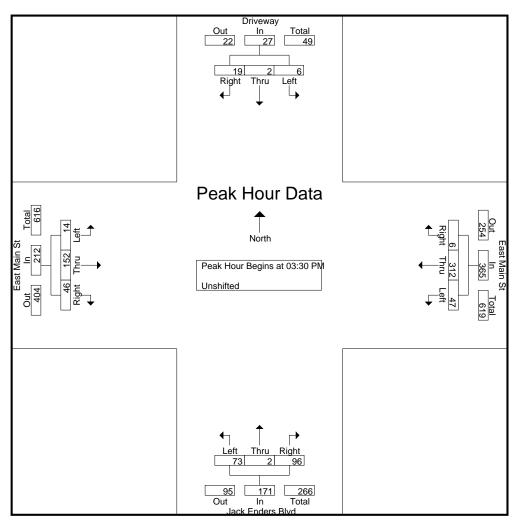


Phone: 703 914-4850

File Name: Main Street @ Jack Enders Blvd

Site Code : 0006 Start Date : 5/21/2019

| | | Driv | eway | | | East I | Main St | | | Jack En | ders Bl | /d | | | | | |
|--|------|------|-------|------------|------|--------|---------|------------|------|---------|---------|------------|------|------|-------|------------|------------|
| | | From | North | | | Fron | n East | | | From | South | | | | | | |
| Start Time | Left | Thru | Right | App. Total | Left | Thru | Right | App. Total | Left | Thru | Right | App. Total | Left | Thru | Right | App. Total | Int. Total |
| Peak Hour Analysis From 03:30 PM to 06:15 PM - Peak 1 of 1 | | | | | | | | | | | | | | | | | |
| Peak Hour for Entire Intersection Begins at 03:30 PM | | | | | | | | | | | | | | | | | |
| 03:30 PM | 2 | 0 | 6 | 8 | 8 | 66 | 1 | 75 | 39 | 1 | 54 | 94 | 4 | 42 | 20 | 66 | 243 |
| 03:45 PM | 1 | 1 | 3 | 5 | 9 | 79 | 3 | 91 | 11 | 1 | 16 | 28 | 3 | 37 | 8 | 48 | 172 |
| 04:00 PM | 2 | 0 | 8 | 10 | 19 | 77 | 2 | 98 | 11 | 0 | 17 | 28 | 4 | 37 | 8 | 49 | 185 |
| 04:15 PM | 1 | 1 | 2 | 4 | 11 | 90 | 0 | 101 | 12 | 0 | 9 | 21 | 3 | 36 | 10 | 49 | 175 |
| Total Volume | 6 | 2 | 19 | 27 | 47 | 312 | 6 | 365 | 73 | 2 | 96 | 171 | 14 | 152 | 46 | 212 | 775 |
| % App. Total | 22.2 | 7.4 | 70.4 | | 12.9 | 85.5 | 1.6 | | 42.7 | 1.2 | 56.1 | | 6.6 | 71.7 | 21.7 | | |
| PHF | .750 | .500 | .594 | .675 | .618 | .867 | .500 | .903 | .468 | .500 | .444 | .455 | .875 | .905 | .575 | .803 | .797 |



MCV Associates, Inc. 4605-C Pinecrest off Park Dr **Alexandria, VA - 22312**Phone: 703 914-4850

File Name: US 340 @ Main Street

Site Code: 0005 Start Date : 5/21/2019

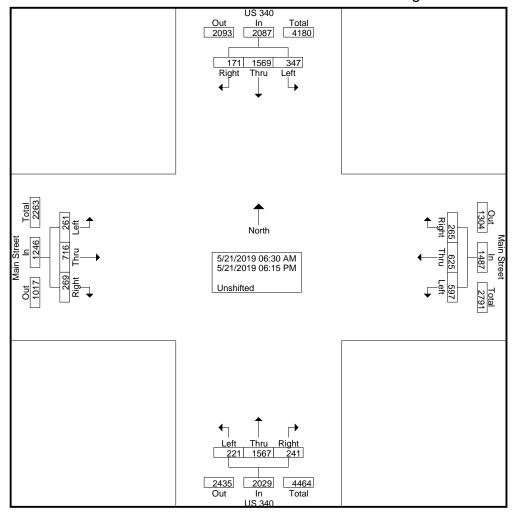
| Crauna | Drintad | Unshifted |
|--------|----------|-----------|
| Groups | Printea- | Unshiitea |

| | US 340 Main Street | | | | | | | | | | PIIII | | US 34 | | | | M | ain St | 1 | | | | |
|-------------|--------------------|-------|-------|-----------|------------|------|------|----------|---------|------------|-------|------|-------|---------|------------|------|---------|--------|---------|------------|--------------|--------------|------------|
| | | om No | | From East | | | | | | | om So | | | | rom W | | | | | | | | |
| Start Time | Left | Thru | Right | U Turns | App. Total | Left | Thru | Right | U Turns | App. Total | Left | Thru | Right | U Turns | App. Total | Left | Thru | Right | U Turns | App. Total | Exclu. Total | Inclu. Total | Int. Total |
| 06:30 AM | 18 | 42 | 2 | 0 | 62 | 10 | 4 | 3 | 0 | 17 | 2 | 57 | 2 | 0 | 61 | 12 | 23 | 3 | 0 | 38 | 0 | 178 | 178 |
| 06:45 AM | 27 | 43 | 5 | 0 | 75 | 9 | 14 | 2 | 0 | 25 | 5 | 69 | 9 | 0 | 83 | 8 | 37 | 9 | 0 | 54 | 0 | 237 | 237 |
| Total | 45 | 85 | 7 | 0 | 137 | 19 | 18 | 5 | 0 | 42 | 7 | 126 | 11 | 0 | 144 | 20 | 60 | 12 | 0 | 92 | 0 | 415 | 415 |
| 07:00 AM | 16 | 60 | 9 | 0 | 85 | 18 | 17 | 5 | 0 | 40 | 4 | 55 | 9 | 0 | 68 | 9 | 27 | 6 | 0 | 42 | 0 | 235 | 235 |
| 07:15 AM | 15 | 58 | 4 | 0 | 77 | 11 | 17 | 2 | 0 | 30 | 5 | 73 | 15 | 0 | 93 | 4 | 30 | 9 | 0 | 43 | 0 | 243 | 243 |
| 07:30 AM | 18 | 51 | 17 | 0 | 86 | 12 | 41 | 8 | 0 | 61 | 11 | 73 | 8 | 0 | 92 | 10 | 28 | 2 | 0 | 40 | 0 | 279 | 279 |
| 07:45 AM | 21 | 60 | 17 | 0 | 98 | 15 | 39 | 7 | 0 | 61 | 13 | 75 | 15 | 0 | 103 | 20 | 41 | 10 | 0 | 71 | 0 | 333 | 333 |
| Total | 70 | 229 | 47 | 0 | 346 | 56 | 114 | 22 | 0 | 192 | 33 | 276 | 47 | 0 | 356 | 43 | 126 | 27 | 0 | 196 | 0 | 1090 | 1090 |
| 08:00 AM | 16 | 84 | 7 | 0 | 107 | 18 | 33 | 10 | 0 | 61 | 10 | 72 | 13 | 0 | 95 | 15 | 61 | 16 | 0 | 92 | 0 | 355 | 355 |
| 08:15 AM | 13 | 38 | 2 | 0 | 53 | 16 | 13 | 8 | 0 | 37 | 8 | 68 | 9 | 0 | 85 | 9 | 45 | 11 | 0 | 65 | 0 | 240 | 240 |
| 08:30 AM | 13 | 47 | 4 | 0 | 64 | 15 | 15 | 12 | 0 | 42 | 2 | 67 | 9 | 0 | 78 | 8 | 29 | 14 | 0 | 51 | 0 | 235 | 235 |
| 08:45 AM | 13 | 50 | 6 | 0 | 69 | 14 | 16 | 8_ | 0 | 38 | 8 | 65 | 11_ | 0 | 84 | 7 | 25 | 11 | 0 | 43 | 0 | 234 | 234 |
| Total | 55 | 219 | 19 | 0 | 293 | 63 | 77 | 38 | 0 | 178 | 28 | 272 | 42 | 0 | 342 | 39 | 160 | 52 | 0 | 251 | 0 | 1064 | 1064 |
| 09:00 AM | 21 | 31 | 4 | 0 | 56 | 14 | 16 | 7 | 0 | 37 | 8 | 52 | 12 | 0 | 72 | 10 | 18 | 6 | 0 | 34 | 0 | 199 | 199 |
| 09:15 AM | 12 | 31 | 8 | 0 | 51 | 15 | 22 | 14 | 0 | 51 | 8 | 63 | 11 | 0 | 82 | 11 | 26 | 9 | 0 | 46 | 0 | 230 | 230 |
| | ' | | | | | | | | | | | | | | | ' | | | | | | | |
| Total | 33 | 62 | 12 | 0 | 107 | 29 | 38 | 21 | 0 | 88 | 16 | 115 | 23 | 0 | 154 | 21 | 44 | 15 | 0 | 80 | 0 | 429 | 429 |
| | | | | | | | | | | | | | | | | | | | | | | | |
| 03:30 PM | 17 | 87 | 6 | 0 | 110 | 26 | 25 | 22 | 0 | 73 | 9 | 75 | 8 | 0 | 92 | 16 | 35 | 16 | 0 | 67 | 0 | 342 | 342 |
| 03:45 PM | 12 | 73 | 8 | 0 | 93 | 43 | 27 | 17 | 0 | 73 87 | 13 | 63 | 10 | 0 | 86 | 14 | 23 | 15 | 0 | 52 | 0 | 318 | 318 |
| Total | 29 | 160 | 14 | 0 | 203 | 69 | 52 | 39 | 0 | 160 | 22 | 138 | 18 | 0 | 178 | 30 | <u></u> | 31 | 0 | 119 | 0 | 660 | 660 |
| | | | | | | | | | | | | | | - | | | | | | | | | |
| 04:00 PM | 17 | 80 | 7 | 0 | 104 | 42 | 26 | 16 | 0 | 84 | 17 | 64 | 11 | 0 | 92 | 12 | 31 | 15 | 0 | 58 | 0 | 338 | 338 |
| 04:15 PM | 11 | 95 | 1 | 0 | 107 | 44 | 27 | 22 | 0 | 93 | 13 | 64 | 12 | 0 | 89 | 16 | 29 | 17 | 0 | 62 | 0 | 351 | 351 |
| 04:30 PM | 10 | 95 | 6 | 0 | 111 | 45 | 46 | 19 | 0 | 110 | 9 | 65 | 10 | 0 | 84 | 14 | 27 | 7 | 0 | 48 | 0 | 353 | 353 |
| 04:45 PM | 17 | 96 | 11 | 0 | 124 | 33 | 33 | 14 71 | 0 | 80 | 10 | 65 | 5 | 0 | 80 | 13 | 25 | 12 | 0 | 50 | 0 | 334 | 334 |
| Total | 55 | 366 | 25 | 0 | 446 | 164 | 132 | /1 | 0 | 367 | 49 | 258 | 38 | 0 | 345 | 55 | 112 | 51 | 0 | 218 | 0 | 1376 | 1376 |
| 05:00 PM | 7 | 75 | 6 | 0 | 88 | 44 | 40 | 16 | 0 | 100 | 8 | 76 | 12 | 0 | 96 | 8 | 36 | 15 | 0 | 59 | 0 | 343 | 343 |
| 05:15 PM | 13 | 91 | 9 | 0 | 113 | 34 | 27 | 12 | 0 | 73 | 15 | 56 | 4 | 0 | 75 | 14 | 28 | 16 | 0 | 58 | 0 | 319 | 319 |
| 05:30 PM | 12 | 89 | 5 | 0 | 106 | 26 | 37 | 10 | 0 | 73 | 10 | 74 | 12 | 0 | 96 | 10 | 24 | 8 | 0 | 42 | 0 | 317 | 317 |
| 05:45 PM | 14 | 79 | 7 | 0 | 100 | 38 | 39 | 13 | 0 | 90 | 13 | 74 | 13 | 0 | 100 | 8 | 26 | 16 | 0 | 50 | 0 | 340 | 340 |
| Total | 46 | 334 | 27 | 0 | 407 | 142 | 143 | 51 | 0 | 336 | 46 | 280 | 41 | 0 | 367 | 40 | 114 | 55 | 0 | 209 | 0 | 1319 | 1319 |
| 06:00 PM | 8 | 57 | 13 | 0 | 78 | 32 | 28 | 10 | 0 | 70 | 10 | 51 | 8 | 0 | 69 | 6 | 24 | 12 | 0 | 42 | 0 | 259 | 259 |
| 06:15 PM | 6 | 57 | 7 | 0 | 70 | 23 | 23 | 8 | Ō | 54 | 10 | 51 | 13 | 0 | 74 | 7 | 18 | 14 | Ō | 39 | 0 | 237 | 237 |
| Grand Total | 347 | 1569 | 171 | 0 | 2087 | 597 | 625 | 265 | 0 | 1487 | 221 | 1567 | 241 | 0 | 2029 | 261 | 716 | 269 | 0 | 1246 | 0 | 6849 | 6849 |
| Apprch % | 16.6 | 75.2 | 8.2 | | | 40.1 | 42 | 17.8 | | | 10.9 | 77.2 | 11.9 | | | 20.9 | 57.5 | 21.6 | | | | | |
| Total % | 5.1 | 22.9 | 2.5 | | 30.5 | 8.7 | 9.1 | 3.9 | | 21.7 | 3.2 | 22.9 | 3.5 | | 29.6 | 3.8 | 10.5 | 3.9 | | 18.2 | 0 | 100 | |

Phone: 703 914-4850

File Name: US 340 @ Main Street

Site Code : 0005 Start Date : 5/21/2019

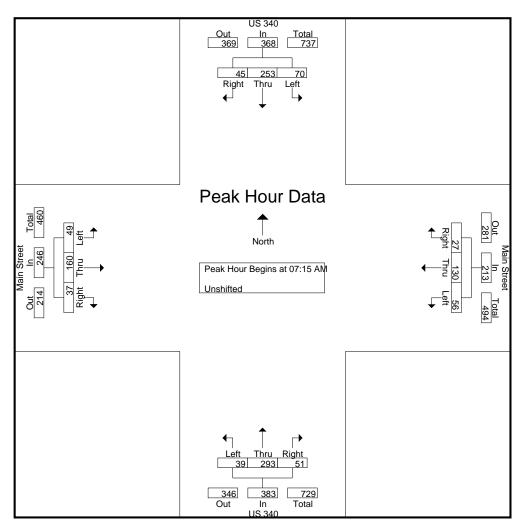


MCV Associates, Inc. 4605-C Pinecrest off Park Dr **Alexandria, VA - 22312**Phone: 703 914-4850

File Name: US 340 @ Main Street

Site Code: 0005 Start Date : 5/21/2019

| | | | 340 | | | | Street | | | | 340 | | | | Street | | |
|-----------------|------------|-----------|----------|------------|--------|------|--------|------------|------|------|-------|------------|------|------|--------|------------|------------|
| | | From | North_ | | | Fron | n East | | | From | South | | | Fron | n West | | |
| Start Time | Left | Thru | Right | App. Total | Left | Thru | Right | App. Total | Left | Thru | Right | App. Total | Left | Thru | Right | App. Total | Int. Total |
| Peak Hour Analy | ysis Fror | n 06:30 | AM to 0 | 9:15 AM - | Peak 1 | of 1 | | | | | | | | | | | |
| Peak Hour for E | ntire Inte | ersection | n Begins | at 07:15 | AM | | | | | | | | | | | | |
| 07:15 AM | 15 | 58 | 4 | 77 | 11 | 17 | 2 | 30 | 5 | 73 | 15 | 93 | 4 | 30 | 9 | 43 | 243 |
| 07:30 AM | 18 | 51 | 17 | 86 | 12 | 41 | 8 | 61 | 11 | 73 | 8 | 92 | 10 | 28 | 2 | 40 | 279 |
| 07:45 AM | 21 | 60 | 17 | 98 | 15 | 39 | 7 | 61 | 13 | 75 | 15 | 103 | 20 | 41 | 10 | 71 | 333 |
| 08:00 AM | 16 | 84 | 7 | 107 | 18 | 33 | 10 | 61 | 10 | 72 | 13 | 95 | 15 | 61 | 16 | 92 | 355 |
| Total Volume | 70 | 253 | 45 | 368 | 56 | 130 | 27 | 213 | 39 | 293 | 51 | 383 | 49 | 160 | 37 | 246 | 1210 |
| % App. Total | 19 | 68.8 | 12.2 | | 26.3 | 61 | 12.7 | | 10.2 | 76.5 | 13.3 | | 19.9 | 65 | 15 | | |
| PHF | .833 | .753 | .662 | .860 | .778 | .793 | .675 | .873 | .750 | .977 | .850 | .930 | .613 | .656 | .578 | .668 | .852 |

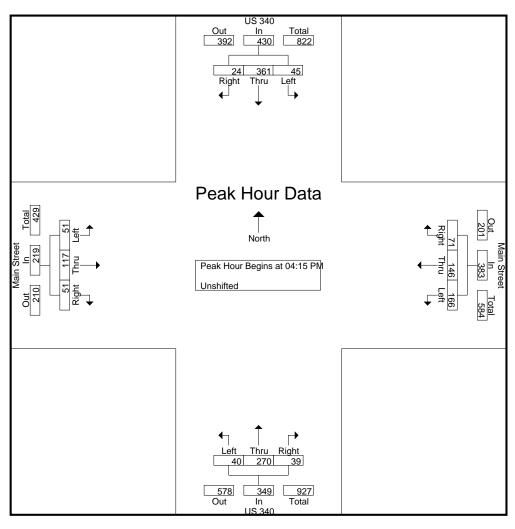


Phone: 703 914-4850

File Name: US 340 @ Main Street

Site Code : 0005 Start Date : 5/21/2019

| | | US | 340 | | | Main | Street | | | US | 340 | | | Main | Street | | |
|-----------------|------------|----------|---------|------------|--------|------|--------|------------|------|------|-------|------------|------|------|--------|------------|------------|
| | | From | North | | | Fron | n East | | | From | South | | | From | n West | | |
| Start Time | Left | Thru | Right | App. Total | Left | Thru | Right | App. Total | Left | Thru | Right | App. Total | Left | Thru | Right | App. Total | Int. Total |
| Peak Hour Analy | ysis Fron | า 03:30 | PM to 0 | 6:15 PM - | Peak 1 | of 1 | | | | | | | | | | | |
| Peak Hour for E | ntire Inte | rsection | Begins | at 04:15 | PM | | | | | | | | | | | | |
| 04:15 PM | 11 | 95 | 1 | 107 | 44 | 27 | 22 | 93 | 13 | 64 | 12 | 89 | 16 | 29 | 17 | 62 | 351 |
| 04:30 PM | 10 | 95 | 6 | 111 | 45 | 46 | 19 | 110 | 9 | 65 | 10 | 84 | 14 | 27 | 7 | 48 | 353 |
| 04:45 PM | 17 | 96 | 11 | 124 | 33 | 33 | 14 | 80 | 10 | 65 | 5 | 80 | 13 | 25 | 12 | 50 | 334 |
| 05:00 PM | 7 | 75 | 6 | 88 | 44 | 40 | 16 | 100 | 8 | 76 | 12 | 96 | 8 | 36 | 15 | 59 | 343 |
| Total Volume | 45 | 361 | 24 | 430 | 166 | 146 | 71 | 383 | 40 | 270 | 39 | 349 | 51 | 117 | 51 | 219 | 1381 |
| % App. Total | 10.5 | 84 | 5.6 | | 43.3 | 38.1 | 18.5 | | 11.5 | 77.4 | 11.2 | | 23.3 | 53.4 | 23.3 | | |
| PHF | .662 | .940 | .545 | .867 | .922 | .793 | .807 | .870 | .769 | .888 | .813 | .909 | .797 | .813 | .750 | .883 | .978_ |



MCV Associates, Inc. 4605-C Pinecrest off Park Dr **Alexandria, VA - 22312**Phone: 703 914-4850

File Name: US 340 @ South Church St

Site Code: 00003 Start Date : 5/21/2019

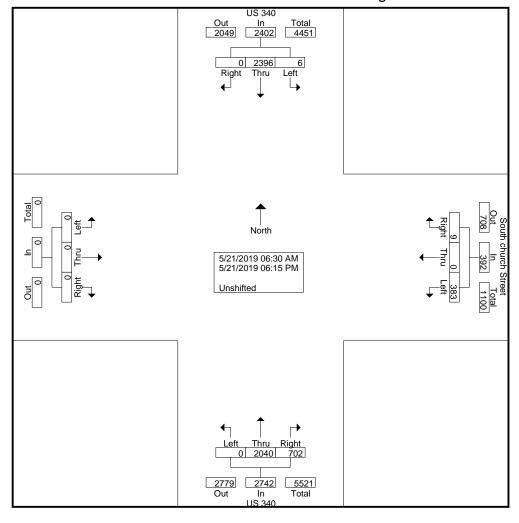
| | Groups | Printed- | Unshifted |
|--|--------|----------|-----------|
|--|--------|----------|-----------|

| | | US | 340 | | Sol | uth chu | rch Str | | SPIIII | ea- Uns US | | | | | | | 1 | | |
|----------------------|------------|------------|-------|---------|----------|---------|---------|---------|--------|---------------|----------|---------|------|------|-------|---------|--------------|--------------|------------|
| | | From | | | 300 | From | | CCI | | From | | | | From | West | | | | |
| Start Time | Left | Thru | Right | U Turns | Left | Thru | Right | U Turns | Left | Thru | Right | U Turns | Left | Thru | Right | U Turns | Exclu. Total | Inclu. Total | Int. Total |
| 06:30 AM | 0 | 56 | 0 | 0 | 3 | 0 | 0 | 0 | 0 | 66 | 34 | 0 | 0 | 0 | 0 | 0 | 0 | 159 | 159 |
| 06:45 AM | 1 | 56 | 0 | 0 | 8 | 0 | 0 | 0 | 0 | 79 | 54 | 0 | 0 | 0 | 0 | 0 | 0 | 198 | 198 |
| Total | 1 | 112 | 0 | 0 | 11 | 0 | 0 | 0 | 0 | 145 | 88 | 0 | 0 | 0 | 0 | 0 | 0 | 357 | 357 |
| 1 | | | _ | | | _ | _ | _ | | | | _ | | _ | _ | _ | | | |
| 07:00 AM | 1 | 94 | 0 | 0 | 15 | 0 | 0 | 0 | 0 | 75 | 43 | 0 | 0 | 0 | 0 | 0 | 0 | 228 | 228 |
| 07:15 AM 07:30 AM | 0 | 78 76 | 0 | 0 0 | 11 13 | 0 | 1 0 | 0 | 0 | 99 105 | 50 49 | 0 0 | 0 | 0 | 0 | 0 | 0 | 239 243 | 239 243 |
| 07:30 AM 07:45 AM | 0 | 76 80 | 0 | 0 | 11 | 0 | 0 | 0 | 0 | 122 | 49 45 | 0 | 0 | 0 | 0 | 0 | 0 | 243 258 | 243 258 |
| Total | 1 | 328 | 0 | 0 | 50 | 0 | 1 | 0 | 0 | 401 | 187 | 0 | 0 | 0 | 0 | 0 | 0 | 968 | 968 |
| rotar | • | 020 | Ü | Ü | , 00 | Ŭ | • | Ü | | 101 | 107 | Ü | | Ü | Ū | Ŭ | | 000 | 000 |
| 08:00 AM | 1 | 122 | 0 | 0 | 4 | 0 | 0 | 0 | 0 | 93 | 48 | 0 | 0 | 0 | 0 | 0 | 0 | 268 | 268 |
| 08:15 AM | 0 | 62 | 0 | 0 | 9 | 0 | 0 | 0 | 0 | 91 | 36 | 0 | 0 | 0 | 0 | 0 | 0 | 198 | 198 |
| 08:30 AM | 0 | 63 | 0 | 0 | 10 | 0 | 0 | 0 | 0 | 81 | 38 | 0 | 0 | 0 | 0 | 0 | 0 | 192 | 192 |
| 08:45 AM | 0 | 61 | 0 | 0 | 9 | 0 | 0 | 0 | 0 | 80 | 31 | 0 | 0 | 0 | 0 | 0 | 0 | 181 | 181 |
| Total | 1 | 308 | 0 | 0 | 32 | 0 | 0 | 0 | 0 | 345 | 153 | 0 | 0 | 0 | 0 | 0 | 0 | 839 | 839 |
| 09:00 AM | 0 | 55 | 0 | 0 | 7 | 0 | 0 | 0 | 0 | 67 | 28 | 0 | 0 | 0 | 0 | 0 | 0 | 157 | 157 |
| 09:00 AM | 1 | 47 | 0 | 0 | 8 | 0 | 0 | 0 | 0 | 82 | 23 | 0 | 0 | 0 | 0 | 0 | 0 | 161 | 161 |
| 05.15 AW | | 7, | U | O | | O | O | U | 0 | 02 | 20 | O | | U | U | U | , 0 | 101 | 101 |
| Total | 1 | 102 | 0 | 0 | 15 | 0 | 0 | 0 | 0 | 149 | 51 | 0 | 0 | 0 | 0 | 0 | 0 | 318 | 318 |
| | | | | | | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | | | |
| | _ | | _ | _ | | _ | _ | | ء ا | | | | | _ | _ | _ | | | |
| 03:30 PM | 0 | 150 | 0 | 0 | 26 | 0 | 0 | 0 | 0 | 85 | 16 | 0 | 0 | 0 | 0 | 0 | 0 | 277 | 277 |
| 03:45 PM Total | <u>1</u> 1 | 122 272 | 0 | 0 | 17 43 | 0 | 2 | 0 | 0 | 89 174 | 27 43 | 0 | 0 | 0 | 0 | 0 | 0 | 258 535 | 258 535 |
| Total | ' | 212 | U | U | 43 | U | 2 | U | 0 | 174 | 43 | U | 0 | U | U | U | 1 0 | 555 | 555 |
| 04:00 PM | 0 | 135 | 0 | 0 | 20 | 0 | 1 | 0 | 0 | 74 | 22 | 0 | 0 | 0 | 0 | 0 | 0 | 252 | 252 |
| 04:15 PM | 0 | 148 | 0 | 0 | 22 | 0 | 0 | 0 | 0 | 94 | 17 | 0 | 0 | 0 | 0 | 0 | 0 | 281 | 281 |
| 04:30 PM | 1 | 147 | 0 | 0 | 31 | 0 | 0 | 0 | 0 | 71 | 23 | 0 | 0 | 0 | 0 | 0 | 0 | 273 | 273 |
| 04:45 PM | 0 | 134 | 0 | 0 | 25 | 0 | 1_ | 0 | 0 | 85 | 13 | 0 | 0 | 0 | 0 | 0 | 0 | 258 | 258 |
| Total | 1 | 564 | 0 | 0 | 98 | 0 | 2 | 0 | 0 | 324 | 75 | 0 | 0 | 0 | 0 | 0 | 0 | 1064 | 1064 |
| 05:00 PM | 0 | 142 | 0 | 0 | 27 | 0 | 2 | 0 | 0 | 96 | 13 | 0 | 0 | 0 | 0 | 0 | 0 | 280 | 280 |
| 05:00 PM 05:15 PM | 0 | 138 | 0 | 0 | 20 | 0 | 0 | 0 | 0 | 80 | 19 | 0 | 0 | 0 | 0 | 0 | 0 | 260 257 | 260 257 |
| 05:30 PM | 0 | 127 | 0 | 0 | 22 | 0 | 0 | 0 | 0 | 87 | 17 | 0 | 0 | 0 | 0 | 0 | 0 | 253 | 253 |
| 05:45 PM | 0 | 120 | 0 | 0 | 32 | Ö | 0 | 0 | ő | 96 | 14 | 0 | 0 | 0 | 0 | Ö | 0 | 262 | 262 |
| Total | 0 | 527 | 0 | 0 | 101 | 0 | 2 | 0 | 0 | 359 | 63 | 0 | 0 | 0 | 0 | 0 | 0 | 1052 | 1052 |
| | | | | | | | | | | | | | | | | | | | |
| 06:00 PM | 0 | 95 | 0 | 0 | 15 | 0 | 1 | 0 | 0 | 79 | 21 | 0 | 0 | 0 | 0 | 0 | 0 | 211 | 211 |
| 06:15 PM | 0 | 88 | 0 | 0 | 18 | 0 | 1 | 0 | 0 | 64 | 21 | 0 | 0 | 0 | 0 | 0 | 0 | 192 | 192 |
| Grand Total | 6 | 2396 | 0 | 0 | 383 | 0 | 9 | 0 | 0 | 2040 | 702 | 0 | 0 | 0 | 0 | 0 | 0 | 5536 | 5536 |
| Apprch % | 0.2 | 99.8 | 0 | | 97.7 | 0 | 2.3 | | 0 | 74.4 | 25.6 | | 0 | 0 | 0 | | | 100 | |
| Total % | 0.1 | 43.3 | 0 | | 6.9 | 0 | 0.2 | | 0 | 36.8 | 12.7 | | 0 | 0 | 0 | | 0 | 100 | |

Phone: 703 914-4850

File Name: US 340 @ South Church St

Site Code : 00003 Start Date : 5/21/2019

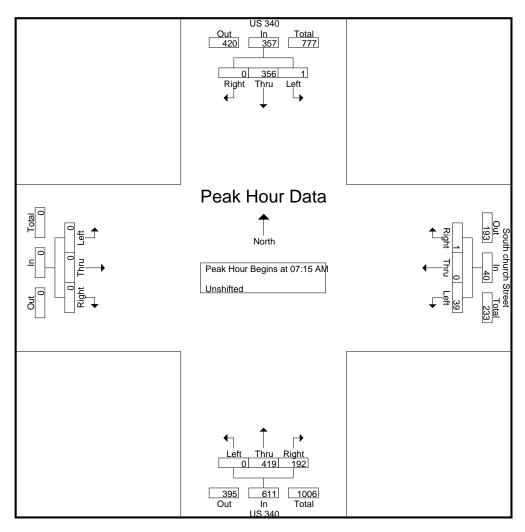


Phone: 703 914-4850

File Name: US 340 @ South Church St

Site Code : 00003 Start Date : 5/21/2019

| | | US | 340 | | S | outh ch | urch Str | reet | | US | 340 | | | | | | |
|-----------------|------------|-----------|----------|------------|--------|---------|----------|------------|------|------|-------|------------|------|------|--------|------------|------------|
| | | From | North | | | Fron | n East | | | From | South | | | Fron | n West | | |
| Start Time | Left | Thru | Right | App. Total | Left | Thru | Right | App. Total | Left | Thru | Right | App. Total | Left | Thru | Right | App. Total | Int. Total |
| Peak Hour Anal | ysis Fror | n 06:30 | AM to 0 | 9:15 AM - | Peak 1 | of 1 | | | | | | | | | | | |
| Peak Hour for E | ntire Inte | ersection | n Begins | at 07:15 | AM | | | | | | | | | | | | |
| 07:15 AM | 0 | 78 | 0 | 78 | 11 | 0 | 1 | 12 | 0 | 99 | 50 | 149 | 0 | 0 | 0 | 0 | 239 |
| 07:30 AM | 0 | 76 | 0 | 76 | 13 | 0 | 0 | 13 | 0 | 105 | 49 | 154 | 0 | 0 | 0 | 0 | 243 |
| 07:45 AM | 0 | 80 | 0 | 80 | 11 | 0 | 0 | 11 | 0 | 122 | 45 | 167 | 0 | 0 | 0 | 0 | 258 |
| 08:00 AM | 1 | 122 | 0 | 123 | 4 | 0 | 0 | 4 | 0 | 93 | 48 | 141 | 0 | 0 | 0 | 0 | 268 |
| Total Volume | 1 | 356 | 0 | 357 | 39 | 0 | 1 | 40 | 0 | 419 | 192 | 611 | 0 | 0 | 0 | 0 | 1008 |
| % App. Total | 0.3 | 99.7 | 0 | | 97.5 | 0 | 2.5 | | 0 | 68.6 | 31.4 | | 0 | 0 | 0 | | |
| PHF | .250 | .730 | .000 | .726 | .750 | .000 | .250 | .769 | .000 | .859 | .960 | .915 | .000 | .000 | .000 | .000 | .940 |

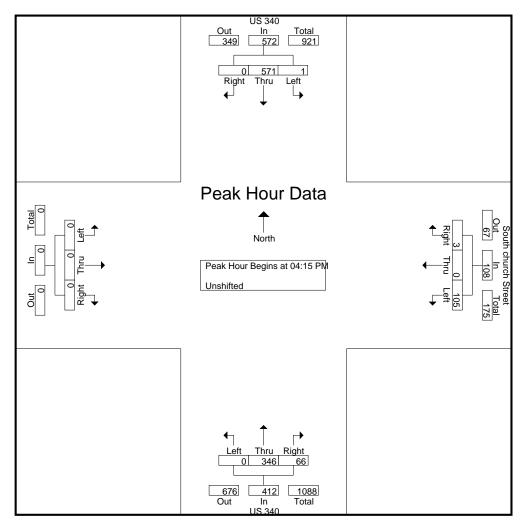


Phone: 703 914-4850

File Name: US 340 @ South Church St

Site Code : 00003 Start Date : 5/21/2019

| | | US | 340 | | S | outh ch | urch Str | eet | | US | 340 | | | | | | |
|-----------------|------------|-----------|---------|------------|--------|---------|----------|------------|------|------|-------|------------|------|------|--------|------------|------------|
| | | From | North | | | Fron | n East | | | From | South | | | From | n West | | |
| Start Time | Left | Thru | Right | App. Total | Left | Thru | Right | App. Total | Left | Thru | Right | App. Total | Left | Thru | Right | App. Total | Int. Total |
| Peak Hour Analy | ysis Fror | n 03:30 | PM to 0 | 6:15 PM - | Peak 1 | of 1 | | | | | | | | | | | |
| Peak Hour for E | ntire Inte | ersection | Begins | at 04:15 | PM | | | | | | | | | | | | |
| 04:15 PM | 0 | 148 | 0 | 148 | 22 | 0 | 0 | 22 | 0 | 94 | 17 | 111 | 0 | 0 | 0 | 0 | 281 |
| 04:30 PM | 1 | 147 | 0 | 148 | 31 | 0 | 0 | 31 | 0 | 71 | 23 | 94 | 0 | 0 | 0 | 0 | 273 |
| 04:45 PM | 0 | 134 | 0 | 134 | 25 | 0 | 1 | 26 | 0 | 85 | 13 | 98 | 0 | 0 | 0 | 0 | 258 |
| 05:00 PM | 0 | 142 | 0 | 142 | 27 | 0 | 2 | 29 | 0 | 96 | 13 | 109 | 0 | 0 | 0 | 0 | 280 |
| Total Volume | 1 | 571 | 0 | 572 | 105 | 0 | 3 | 108 | 0 | 346 | 66 | 412 | 0 | 0 | 0 | 0 | 1092 |
| % App. Total | 0.2 | 99.8 | 0 | | 97.2 | 0 | 2.8 | | 0 | 84 | 16 | | 0 | 0 | 0 | | |
| PHF | .250 | .965 | .000 | .966 | .847 | .000 | .375 | .871 | .000 | .901 | .717 | .928 | .000 | .000 | .000 | .000 | .972 |



| | ሻ | † | r* | Į, | Į. | wJ | • | × | \ | € | × | • |
|------------------------------|-----------|-------|-------|-----------|-------|-----------|-------|-------|----------|-----------|-----------|-----------|
| Movement | NBL | NBT | NBR | SBL | SBT | SBR | SEL | SET | SER | NWL | NWT | NWR |
| Lane Configurations | Ť | f) | | ሻ | 1} | | ሻ | 1> | | ሻ | ₽ | |
| Traffic Volume (veh/h) | 40 | 270 | 39 | 45 | 361 | 24 | 51 | 117 | 51 | 166 | 146 | 71 |
| Future Volume (veh/h) | 40 | 270 | 39 | 45 | 361 | 24 | 51 | 117 | 51 | 166 | 146 | 71 |
| Initial Q (Qb), veh | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Ped-Bike Adj(A_pbT) | 1.00 | | 1.00 | 1.00 | | 1.00 | 1.00 | | 1.00 | 1.00 | | 1.00 |
| Parking Bus, Adj | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Work Zone On Approach | | No | | | No | | | No | | | No | |
| Adj Sat Flow, veh/h/ln | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 |
| Adj Flow Rate, veh/h | 43 | 293 | 42 | 49 | 392 | 26 | 55 | 127 | 55 | 180 | 159 | 77 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Percent Heavy Veh, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Cap, veh/h | 314 | 517 | 74 | 373 | 567 | 38 | 250 | 174 | 75 | 306 | 204 | 99 |
| Arrive On Green | 0.04 | 0.32 | 0.32 | 0.05 | 0.33 | 0.33 | 0.05 | 0.14 | 0.14 | 0.08 | 0.17 | 0.17 |
| Sat Flow, veh/h | 1781 | 1600 | 229 | 1781 | 1735 | 115 | 1781 | 1238 | 536 | 1781 | 1190 | 576 |
| Grp Volume(v), veh/h | 43 | 0 | 335 | 49 | 0 | 418 | 55 | 0 | 182 | 180 | 0 | 236 |
| Grp Sat Flow(s), veh/h/ln | 1781 | 0 | 1829 | 1781 | 0 | 1850 | 1781 | 0 | 1774 | 1781 | 0 | 1767 |
| Q Serve(g_s), s | 1.0 | 0.0 | 9.7 | 1.1 | 0.0 | 12.5 | 1.6 | 0.0 | 6.3 | 5.1 | 0.0 | 8.1 |
| Cycle Q Clear(g_c), s | 1.0 | 0.0 | 9.7 | 1.1 | 0.0 | 12.5 | 1.6 | 0.0 | 6.3 | 5.1 | 0.0 | 8.1 |
| Prop In Lane | 1.00 | 0.0 | 0.13 | 1.00 | 0.0 | 0.06 | 1.00 | 0.0 | 0.30 | 1.00 | 0.0 | 0.33 |
| Lane Grp Cap(c), veh/h | 314 | 0 | 591 | 373 | 0 | 605 | 250 | 0 | 249 | 306 | 0 | 303 |
| V/C Ratio(X) | 0.14 | 0.00 | 0.57 | 0.13 | 0.00 | 0.69 | 0.22 | 0.00 | 0.73 | 0.59 | 0.00 | 0.78 |
| Avail Cap(c_a), veh/h | 379 | 0.00 | 591 | 432 | 0.00 | 605 | 305 | 0.00 | 504 | 306 | 0.00 | 502 |
| HCM Platoon Ratio | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Upstream Filter(I) | 1.00 | 0.00 | 1.00 | 1.00 | 0.00 | 1.00 | 1.00 | 0.00 | 1.00 | 1.00 | 0.00 | 1.00 |
| Uniform Delay (d), s/veh | 14.4 | 0.0 | 17.9 | 13.8 | 0.0 | 18.7 | 21.9 | 0.0 | 26.3 | 22.4 | 0.00 | 25.3 |
| Incr Delay (d2), s/veh | 0.2 | 0.0 | 3.9 | 0.2 | 0.0 | 6.4 | 0.4 | 0.0 | 4.1 | 2.9 | 0.0 | 4.3 |
| Initial Q Delay(d3),s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| %ile BackOfQ(50%),veh/ln | 0.4 | 0.0 | 4.4 | 0.4 | 0.0 | 6.0 | 0.7 | 0.0 | 2.8 | 2.4 | 0.0 | 3.6 |
| Unsig. Movement Delay, s/veh | | 0.0 | 7.7 | 0.4 | 0.0 | 0.0 | 0.1 | 0.0 | 2.0 | ۷.٦ | 0.0 | 5.0 |
| LnGrp Delay(d),s/veh | 14.6 | 0.0 | 21.8 | 14.0 | 0.0 | 25.0 | 22.4 | 0.0 | 30.4 | 25.4 | 0.0 | 29.6 |
| LnGrp LOS | 14.0 B | Α | C C | 14.0 B | Α | 23.0 C | C | Α | C | 23.4 C | Α | 29.0 C |
| <u>_</u> | В | 378 | | В | 467 | | | 237 | | | 416 | |
| Approach Vol, veh/h | | | | | 23.9 | | | | | | | |
| Approach LOS | | 21.0 | | | | | | 28.5 | | | 27.8 C | |
| Approach LOS | | С | | | С | | | С | | | C | |
| Timer - Assigned Phs | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | | | | |
| Phs Duration (G+Y+Rc), s | 9.7 | 27.4 | 11.4 | 15.2 | 9.5 | 27.6 | 9.4 | 17.2 | | | | |
| Change Period (Y+Rc), s | * 6.8 | * 6.8 | * 6.3 | * 6.3 | * 6.8 | * 6.8 | * 6.3 | * 6.3 | | | | |
| Max Green Setting (Gmax), s | * 5 | * 21 | * 5.1 | * 18 | * 5 | * 21 | * 5.1 | * 18 | | | | |
| Max Q Clear Time (g_c+I1), s | 3.1 | 11.7 | 7.1 | 8.3 | 3.0 | 14.5 | 3.6 | 10.1 | | | | |
| Green Ext Time (p_c), s | 0.0 | 1.3 | 0.0 | 0.6 | 0.0 | 1.3 | 0.0 | 0.8 | | | | |
| Intersection Summary | | | | | | | | | | | | |
| HCM 6th Ctrl Delay | | | 25.0 | | | | | | | | | |
| HCM 6th LOS | | | С | | | | | | | | | |
| | | | | | | | | | | | | |

Note:

^{*} HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

| Intersection | | | | | | | | | | | | |
|------------------------|--------|-------|----------|--------|------|------|--------|-------|-------|-----------|-------|-------|
| Int Delay, s/veh | 4.1 | | | | | | | | | | | |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | | 4 | | | 4 | | | र्स | 7 | | 4 | |
| Traffic Vol, veh/h | 14 | 152 | 46 | 47 | 312 | 6 | 73 | 2 | 96 | 6 | 2 | 19 |
| Future Vol, veh/h | 14 | 152 | 46 | 47 | 312 | 6 | 73 | 2 | 96 | 6 | 2 | 19 |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | _ | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | - | _ | - | - | - | - | - | - | 0 | - | | - |
| Veh in Median Storage | e.# - | 0 | _ | _ | 0 | - | - | 0 | - | - | 0 | _ |
| Grade, % | - | 0 | _ | - | 0 | - | - | 0 | - | - | 0 | _ |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 15 | 165 | 50 | 51 | 339 | 7 | 79 | 2 | 104 | 7 | 2 | 21 |
| | | | | | | | | | | | | |
| Major/Minor I | Major1 | | | Major2 | | | Minor1 | | | Minor2 | | |
| Conflicting Flow All | 346 | 0 | 0 | 215 | 0 | 0 | 676 | 668 | 190 | 718 | 690 | 343 |
| Stage 1 | - | - | - | - | - | - | 220 | 220 | - | 445 | 445 | J-10 |
| Stage 2 | _ | _ | _ | _ | _ | _ | 456 | 448 | _ | 273 | 245 | _ |
| Critical Hdwy | 4.12 | _ | - | 4.12 | _ | _ | 7.12 | 6.52 | 6.22 | 7.12 | 6.52 | 6.22 |
| Critical Hdwy Stg 1 | - 1.12 | _ | _ | - 1.12 | _ | _ | 6.12 | 5.52 | U.LL | 6.12 | 5.52 | - |
| Critical Hdwy Stg 2 | _ | _ | _ | | _ | _ | 6.12 | 5.52 | _ | 6.12 | 5.52 | _ |
| Follow-up Hdwy | 2.218 | _ | _ | 2.218 | _ | _ | 3.518 | 4.018 | 3.318 | 3.518 | 4.018 | 3.318 |
| Pot Cap-1 Maneuver | 1213 | _ | _ | 1355 | _ | _ | 367 | 379 | 852 | 344 | 368 | 700 |
| Stage 1 | - 10 | _ | _ | | _ | _ | 782 | 721 | - | 592 | 575 | - |
| Stage 2 | _ | _ | _ | _ | - | _ | 584 | 573 | _ | 733 | 703 | - |
| Platoon blocked, % | | _ | _ | | _ | _ | 301 | 310 | | . 00 | . 00 | |
| Mov Cap-1 Maneuver | 1213 | _ | _ | 1355 | _ | _ | 338 | 356 | 852 | 287 | 346 | 700 |
| Mov Cap-2 Maneuver | - 10 | _ | _ | | _ | _ | 338 | 356 | - | 287 | 346 | - |
| Stage 1 | _ | _ | _ | _ | - | _ | 771 | 711 | _ | 584 | 548 | - |
| Stage 2 | _ | _ | _ | _ | _ | _ | 538 | 546 | _ | 632 | 693 | _ |
| | | | | | | | 300 | 3.3 | | 302 | 300 | |
| Approach | EB | | | WB | | | NB | | | SB | | |
| HCM Control Delay, s | 0.5 | | | 1 | | | 13.8 | | | 12.6 | | |
| HCM LOS | 0.0 | | | | | | В | | | В | | |
| | | | | | | | | | | | | |
| Minor Lane/Major Mvm | nt | NBLn1 | NBLn2 | EBL | EBT | EBR | WBL | WBT | WBR | SBL n1 | | |
| Capacity (veh/h) | | 338 | | 1213 | - | | 1355 | - | - | | | |
| HCM Lane V/C Ratio | | | 0.122 | | _ | | 0.038 | _ | | 0.058 | | |
| HCM Control Delay (s) | | 19 | 9.8 | 8 | 0 | _ | 7.8 | 0 | | | | |
| HCM Lane LOS | | C | 3.0 A | A | A | _ | Α. | A | _ | 12.0 B | | |
| HCM 95th %tile Q(veh) |) | 0.9 | 0.4 | 0 | - | | 0.1 | - | | 0.2 | | |
| HOW JOHN JOHNE Q(VEI) | J | 0.5 | 0.4 | U | | | 0.1 | | | 0.2 | | |

| Intersection | | | | | | |
|--------------------------------------|--------|--------|--------|-------|--------|------|
| Int Delay, s/veh | 2.9 | | | | | |
| | | | | | | |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | N/A | | î» | | | 4 |
| Traffic Vol, veh/h | 105 | 3 | 346 | 66 | 1 | 571 |
| Future Vol, veh/h | 105 | 3 | 346 | 66 | 1 | 571 |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | _ | - | _ | - |
| Veh in Median Storage | | _ | 0 | _ | _ | 0 |
| Grade, % | 0 | _ | 0 | _ | _ | 0 |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| | 2 | 2 | 2 | 2 | 2 | 92 |
| Heavy Vehicles, % | | | | | | |
| Mvmt Flow | 114 | 3 | 376 | 72 | 1 | 621 |
| | | | | | | |
| Major/Minor | Minor1 | N | Major1 | - 1 | Major2 | |
| Conflicting Flow All | 1035 | 412 | 0 | 0 | 448 | 0 |
| Stage 1 | 412 | 412 | - | U | -440 | - |
| Stage 2 | 623 | - | - | _ | - | |
| Critical Hdwy | 6.42 | 6.22 | - | - | 4.12 | - |
| • | | | - | - | | |
| Critical Hdwy Stg 1 | 5.42 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 5.42 | - 0.40 | - | - | - | - |
| Follow-up Hdwy | 3.518 | | - | - | 2.218 | - |
| Pot Cap-1 Maneuver | 257 | 640 | - | - | 1112 | - |
| Stage 1 | 669 | - | - | - | - | - |
| Stage 2 | 535 | - | - | - | - | - |
| Platoon blocked, % | | | - | - | | _ |
| Mov Cap-1 Maneuver | 257 | 640 | - | - | 1112 | - |
| Mov Cap-2 Maneuver | 257 | - | - | - | - | - |
| Stage 1 | 669 | - | - | - | - | - |
| Stage 2 | 534 | - | - | - | - | - |
| - | | | | | | |
| | 1475 | | | | 0.5 | |
| Approach | WB | | NB | | SB | |
| HCM Control Delay, s | 29.6 | | 0 | | 0 | |
| HCM LOS | D | | | | | |
| | | | | | | |
| Minor Lane/Major Mvn | nt | NBT | NRRV | VBLn1 | SBL | SBT |
| | TIC . | INDI | | | | الان |
| Capacity (veh/h) | | - | - | | 1112 | - |
| HCM Lane V/C Ratio | _ | - | - | | 0.001 | - |
| HCM Control Delay (s |) | - | - | -0.0 | 8.2 | 0 |
| | | - | - | D | Α | Α |
| HCM Lane LOS HCM 95th %tile Q(veh | , | | | 2.2 | 0 | / \ |

| | ሻ | † | r* | Ļ | | » J | • | × | \ | € | × | • |
|----------------------------|-------|-------|-------|-------|--------------|------------|-------|-------|----------|-------|-------|-------|
| Lane Group | NBL | NBT | NBR | SBL | SBT | SBR | SEL | SET | SER | NWL | NWT | NWR |
| Lane Configurations | ች | f) | | ች | f. | | ሻ | f) | | ሻ | f) | |
| Traffic Volume (vph) | 96 | 323 | 12 | 3 | 397 | 10 | 33 | 78 | 89 | 181 | 124 | 67 |
| Future Volume (vph) | 96 | 323 | 12 | 3 | 397 | 10 | 33 | 78 | 89 | 181 | 124 | 67 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (ft) | 0 | | 0 | 200 | | 0 | 100 | | 0 | 0 | | 0 |
| Storage Lanes | 1 | | 0 | 1 | | 0 | 1 | | 0 | 1 | | 0 |
| Taper Length (ft) | 25 | | | 100 | | | 25 | | | 25 | | |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt | | 0.995 | | | 0.996 | | | 0.920 | | | 0.947 | |
| Flt Protected | 0.950 | | | 0.950 | | | 0.950 | | | 0.950 | | |
| Satd. Flow (prot) | 1770 | 1853 | 0 | 1770 | 1855 | 0 | 1770 | 1714 | 0 | 1770 | 1764 | 0 |
| Flt Permitted | 0.321 | | | 0.421 | | | 0.628 | | | 0.476 | | |
| Satd. Flow (perm) | 598 | 1853 | 0 | 784 | 1855 | 0 | 1170 | 1714 | 0 | 887 | 1764 | 0 |
| Right Turn on Red | | | Yes | | | Yes | | | Yes | | | Yes |
| Satd. Flow (RTOR) | | 2 | | | 2 | | | 66 | | | 32 | |
| Link Speed (mph) | | 30 | | | 30 | | | 30 | | | 30 | |
| Link Distance (ft) | | 586 | | | 375 | | | 551 | | | 388 | |
| Travel Time (s) | | 13.3 | | | 8.5 | | | 12.5 | | | 8.8 | |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj. Flow (vph) | 104 | 351 | 13 | 3 | 432 | 11 | 36 | 85 | 97 | 197 | 135 | 73 |
| Shared Lane Traffic (%) | | | | | | | | | | | | |
| Lane Group Flow (vph) | 104 | 364 | 0 | 3 | 443 | 0 | 36 | 182 | 0 | 197 | 208 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(ft) | | 12 | | | 12 | | | 12 | | | 12 | |
| Link Offset(ft) | | 0 | | | 0 | | | 0 | | | 0 | |
| Crosswalk Width(ft) | | 16 | | | 16 | | | 16 | | | 16 | |
| Two way Left Turn Lane | | Yes | | | | | | Yes | | | | |
| Headway Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Turning Speed (mph) | 15 | | 9 | 15 | | 9 | 15 | | 9 | 15 | | 9 |
| Number of Detectors | 1 | 2 | | 1 | 2 | | 1 | 2 | | 1 | 2 | |
| Detector Template | Left | Thru | | Left | Thru | | Left | Thru | | Left | Thru | |
| Leading Detector (ft) | 20 | 100 | | 20 | 100 | | 20 | 100 | | 20 | 100 | |
| Trailing Detector (ft) | 0 | 0 | | 0 | 0 | | 0 | 0 | | 0 | 0 | |
| Detector 1 Position(ft) | 0 | 0 | | 0 | 0 | | 0 | 0 | | 0 | 0 | |
| Detector 1 Size(ft) | 20 | 6 | | 20 | 6 | | 20 | 6 | | 20 | 6 | |
| Detector 1 Type | CI+Ex | CI+Ex | | CI+Ex | CI+Ex | | Cl+Ex | CI+Ex | | Cl+Ex | CI+Ex | |
| Detector 1 Channel | | | | | | | | | | | | |
| Detector 1 Extend (s) | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | |
| Detector 1 Queue (s) | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | |
| Detector 1 Delay (s) | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | |
| Detector 2 Position(ft) | | 94 | | | 94 | | | 94 | | | 94 | |
| Detector 2 Size(ft) | | 6 | | | 6 | | | 6 | | | 6 | |
| Detector 2 Type | | CI+Ex | | | CI+Ex | | | CI+Ex | | | CI+Ex | |
| Detector 2 Channel | | | | | | | | | | | | |
| Detector 2 Extend (s) | | 0.0 | | | 0.0 | | | 0.0 | | | 0.0 | |
| Turn Type | pm+pt | NA | | pm+pt | NA | | pm+pt | NA | | pm+pt | NA | |
| Protected Phases | 5 | 2 | | 1 | 6 | | 7 | 4 | | 3 | 8 | |
| Permitted Phases | 2 | | | 6 | | | 4 | | | 8 | | |

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|-------------------------|-------|----------|-----|----------------|----------|-----|-------|--------------|---------------|-------|-------|-----|
| Lane Group | NBL | NBT | NBR | SBL | SBT | SBR | SEL | SET | SER | NWL | NWT | NWR |
| Detector Phase | 5 | 2 | | 1 | 6 | | 7 | 4 | | 3 | 8 | |
| Switch Phase | | | | | | | | | | | | |
| Minimum Initial (s) | 5.0 | 8.0 | | 5.0 | 8.0 | | 5.0 | 8.0 | | 5.0 | 8.0 | |
| Minimum Split (s) | 11.8 | 24.8 | | 11.8 | 24.8 | | 11.3 | 24.3 | | 11.3 | 24.3 | |
| Total Split (s) | 11.9 | 31.2 | | 11.8 | 31.1 | | 11.3 | 24.3 | | 12.7 | 25.7 | |
| Total Split (%) | 14.9% | 39.0% | | 14.8% | 38.9% | | 14.1% | 30.4% | | 15.9% | 32.1% | |
| Maximum Green (s) | 5.1 | 24.4 | | 5.0 | 24.3 | | 5.0 | 18.0 | | 6.4 | 19.4 | |
| Yellow Time (s) | 3.8 | 3.8 | | 3.8 | 3.8 | | 3.5 | 3.5 | | 3.5 | 3.5 | |
| All-Red Time (s) | 3.0 | 3.0 | | 3.0 | 3.0 | | 2.8 | 2.8 | | 2.8 | 2.8 | |
| Lost Time Adjust (s) | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | |
| Total Lost Time (s) | 6.8 | 6.8 | | 6.8 | 6.8 | | 6.3 | 6.3 | | 6.3 | 6.3 | |
| Lead/Lag | Lag | Lead | | Lag | Lead | | Lead | Lag | | Lead | Lag | |
| Lead-Lag Optimize? | Yes | Yes | | Yes | Yes | | Yes | Yes | | Yes | Yes | |
| Vehicle Extension (s) | 3.0 | 3.0 | | 3.0 | 3.0 | | 3.0 | 3.0 | | 3.0 | 3.0 | |
| Recall Mode | None | C-Max | | None | C-Max | | None | None | | None | None | |
| Walk Time (s) | | 7.0 | | | 7.0 | | | 7.0 | | | 7.0 | |
| Flash Dont Walk (s) | | 9.0 | | | 9.0 | | | 9.0 | | | 9.0 | |
| Pedestrian Calls (#/hr) | | 0 | | | 0 | | | 0 | | | 0 | |
| Act Effct Green (s) | 41.5 | 39.5 | | 37.3 | 32.3 | | 17.1 | 12.0 | | 22.1 | 18.3 | |
| Actuated g/C Ratio | 0.52 | 0.49 | | 0.47 | 0.40 | | 0.21 | 0.15 | | 0.28 | 0.23 | |
| v/c Ratio | 0.27 | 0.40 | | 0.01 | 0.59 | | 0.12 | 0.58 | | 0.62 | 0.49 | |
| Control Delay | 15.1 | 16.6 | | 11.3 | 25.1 | | 19.2 | 26.7 | | 30.9 | 27.7 | |
| Queue Delay | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | |
| Total Delay | 15.1 | 16.6 | | 11.3 | 25.1 | | 19.2 | 26.7 | | 30.9 | 27.7 | |
| LOS | В | В | | В | С | | В | С | | С | С | |
| Approach Delay | | 16.2 | | | 25.0 | | | 25.5 | | | 29.2 | |
| Approach LOS | | В | | | С | | | С | | | С | |

Intersection Summary

Area Type: Other

Cycle Length: 80

Actuated Cycle Length: 80

Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 75

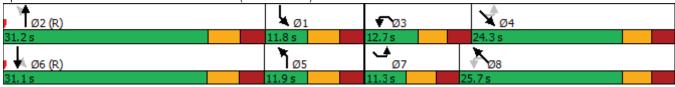
Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.62

Intersection Signal Delay: 23.5 Intersection LOS: C
Intersection Capacity Utilization 68.2% ICU Level of Service C

Analysis Period (min) 15

Splits and Phases: 9: US 340 & VA 7B/VA 7B (E Main Street)



| Intersection | | | | | | | | | | | | |
|------------------------|--------|---------|--------|--------|------|------|--------|------|-------|--------|-------|-------|
| Int Delay, s/veh | 5.5 | | | | | | | | | | | |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | | 4 | | * | ĵ. | | | 4 | 7 | | 4 | |
| Traffic Vol, veh/h | 14 | 123 | 17 | 129 | 307 | 6 | 54 | 2 | 205 | 6 | 2 | 19 |
| Future Vol, veh/h | 14 | 123 | 17 | 129 | 307 | 6 | 54 | 2 | 205 | 6 | 2 | 19 |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | - | - | - | 150 | - | - | 200 | - | 0 | - | - | - |
| Veh in Median Storage | , # - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, % | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 15 | 134 | 18 | 140 | 334 | 7 | 59 | 2 | 223 | 7 | 2 | 21 |
| | | | | | | | | | | | | |
| Major/Minor N | Major1 | | 1 | Major2 | | - 1 | Minor1 | | - 1 | Minor2 | | |
| Conflicting Flow All | 341 | 0 | 0 | 152 | 0 | 0 | 802 | 794 | 143 | 904 | 800 | 338 |
| Stage 1 | - | - | - | - | - | - | 173 | 173 | - | 618 | 618 | - |
| Stage 2 | - | - | - | - | - | - | 629 | 621 | - | 286 | 182 | - |
| Critical Hdwy | 4.12 | - | - | 4.12 | - | - | 7.12 | 6.52 | 6.22 | 7.12 | 6.52 | 6.22 |
| Critical Hdwy Stg 1 | - | - | - | - | - | - | 6.12 | 5.52 | - | 6.12 | 5.52 | - |
| Critical Hdwy Stg 2 | - | - | - | - | - | - | 6.12 | 5.52 | - | 6.12 | 5.52 | - |
| Follow-up Hdwy | 2.218 | - | - | 2.218 | - | - | 3.518 | | | 3.518 | 4.018 | 3.318 |
| Pot Cap-1 Maneuver | 1218 | - | - | 1429 | - | - | 302 | 321 | 905 | 258 | 318 | 704 |
| Stage 1 | - | - | - | - | - | - | 829 | 756 | - | 477 | 481 | - |
| Stage 2 | - | - | - | - | - | - | 470 | 479 | - | 721 | 749 | - |
| Platoon blocked, % | | - | - | | - | - | | | | | | |
| Mov Cap-1 Maneuver | 1218 | - | - | 1429 | - | - | 267 | 286 | 905 | 177 | 283 | 704 |
| Mov Cap-2 Maneuver | - | - | - | - | - | - | 267 | 286 | - | 177 | 283 | - |
| Stage 1 | - | - | - | - | - | - | 818 | 746 | - | 471 | 434 | - |
| Stage 2 | - | - | - | - | - | - | 409 | 432 | - | 535 | 739 | - |
| | | | | | | | | | | | | |
| Approach | EB | | | WB | | | NB | | | SB | | |
| HCM Control Delay, s | 0.7 | | | 2.3 | | | 12.9 | | | 14.8 | | |
| HCM LOS | | | | | | | В | | | В | | |
| | | | | | | | | | | | | |
| Minor Lane/Major Mvm | ıt | NBLn1 I | VBI n2 | EBL | EBT | EBR | WBL | WBT | WBR : | SBI n1 | | |
| Capacity (veh/h) | | 268 | 905 | 1218 | - | | 1429 | - | - | | | |
| HCM Lane V/C Ratio | | | 0.246 | | - | | 0.098 | - | | 0.074 | | |
| HCM Control Delay (s) | | 22.3 | 10.3 | 8 | 0 | - | 7.8 | - | - | | | |
| HCM Lane LOS | | ZZ.3 | В | A | A | _ | Α. | | _ | B | | |
| HCM 95th %tile Q(veh) |) | 0.9 | 1 | 0 | - | _ | 0.3 | _ | _ | 0.2 | | |
| HOW JOHN JUNE Q(VEII) | | 0.5 | 1 | 0 | | | 0.0 | | | 0.2 | | |

| Intersection | | | | | | |
|------------------------|--------|-------|--------|-----------|----------|-------|
| Int Delay, s/veh | 3.6 | | | | | |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | W | | ₽ | | | 4 |
| Traffic Vol, veh/h | 107 | 3 | 436 | 48 | 1 | 608 |
| Future Vol, veh/h | 107 | 3 | 436 | 48 | 1 | 608 |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage | e, # 0 | - | 0 | _ | _ | 0 |
| Grade, % | 0 | _ | 0 | - | - | 0 |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 116 | 3 | 474 | 52 | 1 | 661 |
| WWW.CT IOW | 110 | | ••• | 02 | • | 001 |
| | | | | | | |
| | Minor1 | | Major1 | | Major2 | |
| Conflicting Flow All | 1163 | 500 | 0 | 0 | 526 | 0 |
| Stage 1 | 500 | - | - | - | - | - |
| Stage 2 | 663 | - | - | - | - | - |
| Critical Hdwy | 6.42 | 6.22 | - | - | 4.12 | - |
| Critical Hdwy Stg 1 | 5.42 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - |
| Follow-up Hdwy | 3.518 | 3.318 | - | - | 2.218 | - |
| Pot Cap-1 Maneuver | 215 | 571 | - | - | 1041 | - |
| Stage 1 | 609 | - | - | - | - | - |
| Stage 2 | 512 | - | - | - | - | - |
| Platoon blocked, % | | | - | - | | - |
| Mov Cap-1 Maneuver | 215 | 571 | - | - | 1041 | - |
| Mov Cap-2 Maneuver | 215 | - | - | - | - | - |
| Stage 1 | 609 | - | - | - | - | - |
| Stage 2 | 511 | _ | - | _ | _ | _ |
| | J | | | | | |
| | | | | | | |
| Approach | WB | | NB | | SB | |
| HCM Control Delay, s | 39.6 | | 0 | | 0 | |
| HCM LOS | Е | | | | | |
| | | | | | | |
| Minor Lane/Major Mvm | nt | NBT | NRRV | VBLn1 | SBL | SBT |
| Capacity (veh/h) | ic . | INDI | | 219 | 1041 | - 201 |
| HCM Lane V/C Ratio | | - | - | 0.546 | | |
| HCM Control Delay (s) | | - | - | 39.6 | 8.5 | 0 |
| HCM Lane LOS | | - | - | 39.6 E | 6.5 A | A |
| HCM 95th %tile Q(veh | ١ | - | | 2.9 | 0 | |
| HOW SOUL WILLE CLIVEN |) | - | - | 2.9 | U | - |

Queuing and Blocking Report PM Peak

02/10/2020

Intersection: 27: CSX Rail & Smallwood Lane

| Movement | EB | WB | NB | SB |
|-----------------------|-----|------|-----|------|
| Directions Served | T | T | T | Т |
| Maximum Queue (ft) | 637 | 1021 | 15 | 17 |
| Average Queue (ft) | 128 | 203 | 3 | 3 |
| 95th Queue (ft) | 483 | 769 | 16 | 19 |
| Link Distance (ft) | 701 | 1326 | 776 | 1042 |
| Upstream Blk Time (%) | 0 | | | |
| Queuing Penalty (veh) | 1 | | | |
| Storage Bay Dist (ft) | | | | |
| Storage Blk Time (%) | | | | |
| Queuing Penalty (veh) | | | | |

Zone Summary

Zone wide Queuing Penalty: 1

Berryville Planning Study
PRIME AE - AP
SimTraffic Report
Page 5

02/10/2020

| | ሽ | † | ρ¥ | Į, | † | wJ | • | × | > | ₩ | × | * |
|------------------------------|-------|-------|-------|-------|----------|-------|-------|----------|------|------|------|------|
| Movement | NBL | NBT | NBR | SBL | SBT | SBR | SEL | SET | SER | NWL | NWT | NWR |
| Lane Configurations | 7 | ĵ» | | * | ₽ | | ሻ | ₽ | | 7 | f) | |
| Traffic Volume (veh/h) | 67 | 255 | 66 | 78 | 362 | 18 | 47 | 118 | 84 | 219 | 158 | 126 |
| Future Volume (veh/h) | 67 | 255 | 66 | 78 | 362 | 18 | 47 | 118 | 84 | 219 | 158 | 126 |
| Initial Q (Qb), veh | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Ped-Bike Adj(A_pbT) | 1.00 | | 1.00 | 1.00 | | 1.00 | 1.00 | | 1.00 | 1.00 | | 1.00 |
| Parking Bus, Adj | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Work Zone On Approach | | No | | | No | | | No | | | No | |
| Adj Sat Flow, veh/h/ln | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 |
| Adj Flow Rate, veh/h | 73 | 277 | 72 | 85 | 393 | 20 | 51 | 128 | 91 | 238 | 172 | 137 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Percent Heavy Veh, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Cap, veh/h | 316 | 452 | 117 | 356 | 557 | 28 | 236 | 161 | 114 | 336 | 215 | 171 |
| Arrive On Green | 0.06 | 0.32 | 0.32 | 0.06 | 0.32 | 0.32 | 0.04 | 0.16 | 0.16 | 0.11 | 0.22 | 0.22 |
| Sat Flow, veh/h | 1781 | 1431 | 372 | 1781 | 1764 | 90 | 1781 | 1017 | 723 | 1781 | 964 | 768 |
| Grp Volume(v), veh/h | 73 | 0 | 349 | 85 | 0 | 413 | 51 | 0 | 219 | 238 | 0 | 309 |
| Grp Sat Flow(s),veh/h/ln | 1781 | 0 | 1803 | 1781 | 0 | 1854 | 1781 | 0 | 1740 | 1781 | 0 | 1732 |
| Q Serve(g_s), s | 0.0 | 0.0 | 11.9 | 0.0 | 0.0 | 14.2 | 1.7 | 0.0 | 8.8 | 7.9 | 0.0 | 12.2 |
| Cycle Q Clear(g_c), s | 0.0 | 0.0 | 11.9 | 0.0 | 0.0 | 14.2 | 1.7 | 0.0 | 8.8 | 7.9 | 0.0 | 12.2 |
| Prop In Lane | 1.00 | | 0.21 | 1.00 | | 0.05 | 1.00 | | 0.42 | 1.00 | | 0.44 |
| Lane Grp Cap(c), veh/h | 316 | 0 | 569 | 356 | 0 | 585 | 236 | 0 | 275 | 336 | 0 | 386 |
| V/C Ratio(X) | 0.23 | 0.00 | 0.61 | 0.24 | 0.00 | 0.71 | 0.22 | 0.00 | 0.80 | 0.71 | 0.00 | 0.80 |
| Avail Cap(c_a), veh/h | 338 | 0 | 569 | 378 | 0 | 585 | 282 | 0 | 432 | 336 | 0 | 497 |
| HCM Platoon Ratio | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Upstream Filter(I) | 1.00 | 0.00 | 1.00 | 1.00 | 0.00 | 1.00 | 1.00 | 0.00 | 1.00 | 1.00 | 0.00 | 1.00 |
| Uniform Delay (d), s/veh | 26.6 | 0.0 | 21.1 | 24.8 | 0.0 | 21.9 | 24.2 | 0.0 | 29.4 | 23.2 | 0.0 | 26.7 |
| Incr Delay (d2), s/veh | 0.4 | 0.0 | 4.9 | 0.3 | 0.0 | 7.0 | 0.5 | 0.0 | 5.5 | 6.7 | 0.0 | 7.1 |
| Initial Q Delay(d3),s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| %ile BackOfQ(50%),veh/ln | 1.1 | 0.0 | 5.5 | 1.2 | 0.0 | 6.9 | 0.7 | 0.0 | 3.9 | 3.9 | 0.0 | 5.6 |
| Unsig. Movement Delay, s/veh | | 0.0 | 0.0 | | 0.0 | 0.0 | • | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| LnGrp Delay(d),s/veh | 27.0 | 0.0 | 26.0 | 25.2 | 0.0 | 28.9 | 24.6 | 0.0 | 34.9 | 29.9 | 0.0 | 33.8 |
| LnGrp LOS | С | A | C | C | A | С | С | A | С | C | A | С |
| Approach Vol, veh/h | | 422 | | | 498 | | | 270 | | | 547 | |
| Approach Delay, s/veh | | 26.1 | | | 28.2 | | | 33.0 | | | 32.1 | |
| Approach LOS | | C | | | C C | | | C | | | C | |
| | | | | | | | | | | | U | |
| Timer - Assigned Phs | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 | | | | |
| Phs Duration (G+Y+Rc), s | 10.9 | 29.7 | 14.2 | 17.8 | 10.9 | 29.7 | 9.5 | 22.5 | | | | |
| Change Period (Y+Rc), s | * 6.8 | * 6.8 | * 6.3 | * 6.3 | * 6.8 | * 6.8 | * 6.3 | * 6.3 | | | | |
| Max Green Setting (Gmax), s | * 5 | * 23 | * 7.9 | * 18 | * 5 | * 23 | * 5.1 | * 21 | | | | |
| Max Q Clear Time (g_c+I1), s | 2.0 | 13.9 | 9.9 | 10.8 | 2.0 | 16.2 | 3.7 | 14.2 | | | | |
| Green Ext Time (p_c), s | 0.0 | 1.4 | 0.0 | 0.7 | 0.0 | 1.4 | 0.0 | 1.0 | | | | |
| Intersection Summary | | | | | | | | | | | | |
| HCM 6th Ctrl Delay | | | 29.7 | | | | | | | | | |
| HCM 6th LOS | | | С | | | | | | | | | |
| Notes | | | | | | | | | | | | |

^{*} HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

| Intersection | | | | | | | | | | | | | |
|------------------------|--------|---------|----------|-----------|------|--------|----------|---------|----------|--------|-------|----------|------------|
| Int Delay, s/veh | 26.8 | | | | | | | | | | | | |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR | |
| Lane Configurations | | 4 | | ች | ĵ. | | | 4 | 7 | | 4 | | |
| Traffic Vol, veh/h | 14 | 166 | 177 | 105 | 320 | 6 | 229 | 2 | 260 | 6 | 2 | 19 | |
| Future Vol, veh/h | 14 | 166 | 177 | 105 | 320 | 6 | 229 | 2 | 260 | 6 | 2 | 19 | |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop | |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None | |
| Storage Length | - | - | - | 150 | - | - | 200 | - | 0 | - | - | - | |
| Veh in Median Storage | e, # - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - | |
| Grade, % | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - | |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 | 92 | 92 | 92 | 92 | 92 | 92 | |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | |
| Mvmt Flow | 15 | 180 | 192 | 114 | 348 | 7 | 249 | 2 | 283 | 7 | 2 | 21 | |
| | , | | ., | | | - | | | | | | | |
| Major/Minor I | Major1 | | | Major2 | | - 1 | Minor1 | | | Minor2 | | | |
| Conflicting Flow All | 355 | 0 | 0 | 372 | 0 | 0 | 897 | 889 | 276 | 1029 | 982 | 352 | |
| Stage 1 | - | - | - | - | - | - | 306 | 306 | 210 | 580 | 580 | - | |
| Stage 2 | | _ | | _ | _ | | 591 | 583 | _ | 449 | 402 | _ | |
| Critical Hdwy | 4.12 | | - | 4.12 | - | | 7.12 | 6.52 | 6.22 | 7.12 | 6.52 | 6.22 | |
| Critical Hdwy Stg 1 | 4.12 | _ | | 7.12 | _ | | 6.12 | 5.52 | 0.22 | 6.12 | 5.52 | 0.22 | |
| Critical Hdwy Stg 2 | - | _ | - | - | - | - | 6.12 | 5.52 | | 6.12 | 5.52 | | |
| Follow-up Hdwy | 2.218 | - | - | 2.218 | - | - | 3.518 | | | 3.518 | 4.018 | 3.318 | |
| Pot Cap-1 Maneuver | 1204 | _ | - | 1186 | | _ | 261 | 282 | 763 | 212 | 249 | 692 | |
| Stage 1 | 1204 | _ | | 1100 | _ | - | 704 | 662 | 703 | 500 | 500 | 092 | |
| Stage 1 | - | - | - | - | - | - | 493 | 499 | | 589 | 600 | - | |
| Platoon blocked, % | - | - | - | - | - | - | 433 | 433 | - | 309 | 000 | - | |
| Mov Cap-1 Maneuver | 1204 | - | - | 1186 | - | - | ~ 230 | 251 | 763 | 121 | 222 | 692 | |
| Mov Cap-1 Maneuver | 1204 | - | - | 1100 | - | - | ~ 230 | 251 | 703 | 121 | 222 | 092 | |
| • | - | - | - | - | - | - | 693 | 651 | | 492 | 452 | - | |
| Stage 1 | - | - | | - | - | - | 430 | 451 | | 364 | 590 | - | |
| Stage 2 | - | - | - | - | - | - | 430 | 401 | - | 304 | 590 | - | |
| A I | | | | VA (D | | | ND | | | 0.0 | | | |
| Approach | EB | | | WB | | | NB | | | SB | | | |
| HCM Control Delay, s | 0.3 | | | 2 | | | 68.4 | | | 17.6 | | | |
| HCM LOS | | | | | | | F | | | С | | | |
| | | | | | | | | | | | | | |
| Minor Lane/Major Mvm | nt | NBLn1 N | NBLn2 | EBL | EBT | EBR | WBL | WBT | WBR: | SBLn1 | | | |
| Capacity (veh/h) | | 230 | 763 | 1204 | - | - | 1186 | - | - | 314 | | | |
| HCM Lane V/C Ratio | | 1.092 | | 0.013 | - | - | 0.096 | - | - | 0.093 | | | |
| HCM Control Delay (s) | | 131.4 | 12.5 | 8 | 0 | - | 8.4 | - | - | 17.6 | | | |
| HCM Lane LOS | | F | В | A | A | - | А | - | - | С | | | |
| HCM 95th %tile Q(veh) |) | 11.1 | 1.7 | 0 | - | - | 0.3 | - | - | 0.3 | | | |
| Notes | | | | | | | | | | | | | |
| | 000:4: | ¢. D | lov s | 200d = 01 | 200 | 0 | nutotic. | Net D | ofin - d | *. 41 | maiar | (aluma s | in platace |
| ~: Volume exceeds cap | pacity | \$: De | elay exc | ceeds 30 | JUS | +: Com | putation | ו אסנ ט | etinea | :: All | major | volume | in platoon |

| | ۶ | → | * | • | ← | • | 1 | † | ~ | / | ↓ | ∢ |
|------------------------------|-----------|----------|----------|-----------|----------|-----------|-----------|------|-----------|-----------|----------|------|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | | 4 | | | ₽ | | | ર્ન | 7 | | - 4 | |
| Traffic Volume (veh/h) | 14 | 166 | 177 | 105 | 320 | 6 | 229 | 2 | 260 | 6 | 2 | 19 |
| Future Volume (veh/h) | 14 | 166 | 177 | 105 | 320 | 6 | 229 | 2 | 260 | 6 | 2 | 19 |
| Initial Q (Qb), veh | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Ped-Bike Adj(A_pbT) | 1.00 | | 1.00 | 1.00 | | 1.00 | 1.00 | | 1.00 | 1.00 | | 1.00 |
| Parking Bus, Adj | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Work Zone On Approach | | No | | | No | | | No | | | No | |
| Adj Sat Flow, veh/h/ln | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 |
| Adj Flow Rate, veh/h | 15 | 180 | 192 | 114 | 348 | 7 | 249 | 2 | 283 | 7 | 2 | 21 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Percent Heavy Veh, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Cap, veh/h | 60 | 214 | 217 | 247 | 499 | 10 | 483 | 4 | 433 | 99 | 28 | 298 |
| Arrive On Green | 0.27 | 0.27 | 0.27 | 0.27 | 0.27 | 0.27 | 0.27 | 0.27 | 0.27 | 0.26 | 0.26 | 0.26 |
| Sat Flow, veh/h | 24 | 782 | 794 | 1010 | 1827 | 37 | 1768 | 14 | 1585 | 384 | 110 | 1151 |
| Grp Volume(v), veh/h | 387 | 0 | 0 | 114 | 0 | 355 | 251 | 0 | 283 | 30 | 0 | 0 |
| Grp Sat Flow(s),veh/h/ln | 1601 | 0 | 0 | 1010 | 0 | 1864 | 1782 | 0 | 1585 | 1644 | 0 | 0 |
| Q Serve(g_s), s | 4.4 | 0.0 | 0.0 | 0.4 | 0.0 | 11.9 | 8.3 | 0.0 | 11.0 | 1.0 | 0.0 | 0.0 |
| Cycle Q Clear(g_c), s | 16.3 | 0.0 | 0.0 | 16.7 | 0.0 | 11.9 | 8.3 | 0.0 | 11.0 | 1.0 | 0.0 | 0.0 |
| Prop In Lane | 0.04 | | 0.50 | 1.00 | | 0.02 | 0.99 | | 1.00 | 0.23 | | 0.70 |
| Lane Grp Cap(c), veh/h | 491 | 0 | 0 | 247 | 0 | 509 | 487 | 0 | 433 | 426 | 0 | 0 |
| V/C Ratio(X) | 0.79 | 0.00 | 0.00 | 0.46 | 0.00 | 0.70 | 0.52 | 0.00 | 0.65 | 0.07 | 0.00 | 0.00 |
| Avail Cap(c_a), veh/h | 504 | 0 | 0 | 255 | 0 | 523 | 487 | 0 | 433 | 426 | 0 | 0 |
| HCM Platoon Ratio | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Upstream Filter(I) | 1.00 | 0.00 | 0.00 | 1.00 | 0.00 | 1.00 | 1.00 | 0.00 | 1.00 | 1.00 | 0.00 | 0.00 |
| Uniform Delay (d), s/veh | 24.0 | 0.0 | 0.0 | 24.7 | 0.0 | 22.7 | 21.3 | 0.0 | 22.3 | 19.4 | 0.0 | 0.0 |
| Incr Delay (d2), s/veh | 8.0 | 0.0 | 0.0 | 1.3 | 0.0 | 3.9 | 3.9 | 0.0 | 7.5 | 0.3 | 0.0 | 0.0 |
| Initial Q Delay(d3),s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| %ile BackOfQ(50%),veh/ln | 6.8 | 0.0 | 0.0 | 1.8 | 0.0 | 5.4 | 3.8 | 0.0 | 4.7 | 0.4 | 0.0 | 0.0 |
| Unsig. Movement Delay, s/veh | | 0.0 | 0.0 | 26.0 | 0.0 | 06.6 | 05.0 | 0.0 | 20.0 | 10.0 | 0.0 | 0.0 |
| LnGrp Delay(d),s/veh | 32.0 C | 0.0 | 0.0 A | 26.0 C | 0.0 A | 26.6 C | 25.2 C | 0.0 | 29.8 C | 19.8 B | 0.0 | 0.0 |
| LnGrp LOS | U | A | A | U | | U | U | A | U | Б | A 20 | A |
| Approach Vol, veh/h | | 387 | | | 469 | | | 534 | | | 30 | |
| Approach Delay, s/veh | | 32.0 | | | 26.5 | | | 27.6 | | | 19.8 | |
| Approach LOS | | С | | | С | | | С | | | В | |
| Timer - Assigned Phs | | 2 | | 4 | | 6 | | 8 | | | | |
| Phs Duration (G+Y+Rc), s | | 23.5 | | 23.5 | | 22.5 | | 23.5 | | | | |
| Change Period (Y+Rc), s | | 4.5 | | 4.5 | | 4.5 | | 4.5 | | | | |
| Max Green Setting (Gmax), s | | 19.0 | | 19.5 | | 18.0 | | 19.5 | | | | |
| Max Q Clear Time (g_c+l1), s | | 13.0 | | 18.3 | | 3.0 | | 18.7 | | | | |
| Green Ext Time (p_c), s | | 1.1 | | 0.4 | | 0.1 | | 0.3 | | | | |
| Intersection Summary | | | | | | | | | | | | |
| HCM 6th Ctrl Delay | | | 28.3 | | | | | | | | | |
| HCM 6th LOS | | | С | | | | | | | | | |

| 1 | | | | | |
|--------|--|---|---|---|---|
| 4 | | | | | |
| WBL | WBR | NBT | NBR | SBL | SBT |
| 144 | | f) | | | र्स |
| 117 | 3 | 370 | 71 | 1 | 637 |
| 117 | 3 | 370 | 71 | 1 | 637 |
| 0 | 0 | 0 | 0 | 0 | 0 |
| Stop | Stop | Free | Free | Free | Free |
| - | None | - | None | - | None |
| 0 | - | - | - | - | - |
| e, # 0 | - | 0 | - | - | 0 |
| 0 | - | 0 | - | - | 0 |
| 92 | 92 | 92 | 92 | 92 | 92 |
| | | | | | 2 |
| | | | | | 692 |
| 121 | Ū | 102 | | • | 002 |
| | | | | | |
| Minor1 | N | //ajor1 | 1 | Major2 | |
| | 441 | 0 | 0 | 479 | 0 |
| 441 | - | - | - | - | - |
| 694 | - | - | - | - | - |
| 6.42 | 6.22 | - | - | 4.12 | - |
| 5.42 | - | - | - | - | - |
| 5.42 | - | - | - | - | - |
| | 3.318 | - | - | 2.218 | - |
| | | - | - | | - |
| | - | - | _ | - | - |
| | _ | _ | - | _ | _ |
| | | _ | - | | _ |
| 224 | 616 | | | 1083 | _ |
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| | | | | | _ |
| | | - | - | - | _ |
| 490 | - | - | - | - | - |
| | | | | | |
| WB | | NB | | SB | |
| 40 | | 0 | | 0 | |
| | | | | | |
| Ε | | | | | |
| | | | | | |
| | NIE- | | | 07: | 05- |
| nt | NBT | NBRV | VBLn1 | SBL | SBT |
| | NBT - | - | 228 | 1083 | SBT - |
| nt | NBT - | - | 228 0.572 | 1083 0.001 | - |
| | NBT - - | - | 228 0.572 40 | 1083 | - - 0 |
| nt | NBT - - - | - | 228 0.572 | 1083 0.001 | - |
| | 117 117 0 Stop 0 9, # 0 0 92 127 Minor1 1135 441 694 6.42 5.42 5.42 3.518 224 648 496 224 648 496 WB 40 | WBL WBR 117 3 117 3 0 0 Stop Stop - None 0 9, # 0 92 92 2 2 127 3 Minor1 N 1135 441 441 694 6.42 6.22 5.42 5.42 3.518 3.318 224 616 648 496 224 616 224 648 495 WB 40 | WBL WBR NBT 117 3 370 117 3 370 0 0 0 Stop Stop Free None - 0 0 - 0 92 92 92 2 2 2 127 3 402 Minor1 Major1 Major1 1135 441 0 441 - - 6.42 6.22 - 5.42 - - 5.42 - - 3.518 3.318 - 224 616 - 496 - - 224 616 - 496 - - 648 - - 495 - - WB NB 40 0 | WBL WBR NBT NBR 117 3 370 71 117 3 370 71 0 0 0 0 Stop Stop Free Free - None - None 0 - - - 0 - 0 - 92 92 92 92 2 2 2 2 127 3 402 77 Minor1 Major1 I 1135 441 0 0 441 - - - 642 6.22 - - 5.42 - - - 5.42 - - - 5.42 - - - 3.518 3.318 - - 224 616 - - 496 - - | WBL WBR NBT NBR SBL 117 3 370 71 1 117 3 370 71 1 0 0 0 0 0 Stop Stop Free Free Free - None - - - 0 - - - - 9, # 0 - 0 - - 92 92 92 92 92 12 2 2 2 2 2 127 3 402 77 1 1 Minor1 Major1 Major2 Major2 1 1135 441 0 0 479 441 -< |



Appendix C: Construction and ROW Costs

Appendix C: Summary Costs

| Summary Costs | | | | |
|------------------------------------|---------|---------|----------|----------|
| | B1 | B2 | D1 | D2 |
| Construction Cost | | | | |
| Mainline | \$8,130 | \$7,950 | \$5,280 | \$4,820 |
| Intersection Improvements | \$560 | \$560 | \$430 | \$430 |
| New Traffic Signals | \$600 | \$600 | \$540 | \$540 |
| At Grade Railroad Crossing | \$160 | \$160 | \$0 | \$0 |
| Subtotal: Construction Cost | \$9,460 | \$9,280 | \$6,250 | \$5,790 |
| Total Length (feet) | 5,500 | 5,650 | 3,200 | 3,000 |
| Cost per mile | \$9,080 | \$8,670 | \$10,310 | \$10,200 |
| ROW Costs | | | | |
| ROW - Open Space (SF) | 249,372 | 231,511 | 149,848 | 188,121 |
| ROW - Residence or Business | 93,794 | 102,366 | 51,523 | 60,095 |
| Subtotal: ROW Cost \$ | \$100 | \$100 | \$60 | \$70 |
| Total Cost | \$9,560 | \$9,380 | \$6,310 | \$5,870 |

Appendix: Cost Estimate

Concept Cost by Cost Element

| Item | | | B1 | В2 | D1 | D2 |
|-------|--------------------------|------------|-------------|-------------|-------------|-------------|
| | Length in Feet | | 5,500 | 5,650 | 3,200 | 3,000 |
| 100 | Mobilization | LS | \$280,930 | \$274,360 | \$180,160 | \$163,480 |
| 111 | Clearing and Grubbing | Acre | \$34,520 | \$14,930 | \$27,990 | \$14,930 |
| 120 | Regular Excavation | CY | \$413,630 | \$178,870 | \$335,380 | \$178,870 |
| 505 | Bedding Mat Agg 25 | Ton | \$651,850 | \$669,630 | \$379,260 | \$355,560 |
| 580 | Underdrain UD 1 | LF | \$204,050 | \$209,620 | \$118,720 | \$111,300 |
| 1080 | 8" Pipe | LF | \$165,000 | \$169,500 | \$96,000 | \$90,000 |
| 6751 | Drop Inlet | EA | \$78,200 | \$78,200 | \$78,200 | \$78,200 |
| 9056 | Manhole | LF | \$10,150 | \$10,150 | \$10,150 | \$10,150 |
| 10099 | Aggr Mat 21 B | Ton | \$997,460 | \$1,024,670 | \$580,340 | \$544,070 |
| 10611 | Asphault Conc | Ton | \$1,061,000 | \$1,089,930 | \$617,310 | \$578,730 |
| 10637 | Asphault Surface | Ton | \$1,352,050 | \$1,388,920 | \$786,650 | \$737,480 |
| 12600 | Standard Curb Gut CG 6 | LF | \$7,500 | \$7,500 | \$7,500 | \$7,500 |
| 13294 | Guardrail GR 8 | LF | \$11,290 | \$11,290 | \$11,290 | \$11,290 |
| 13345 | GR 9 Terminal | EA | \$14,470 | \$14,470 | \$14,470 | \$14,470 |
| | Topsoil | CY | \$50,930 | \$52,310 | \$29,630 | \$27,780 |
| 50108 | Sign Panel | SF | \$6,960 | \$6,960 | \$6,960 | \$6,960 |
| 54037 | Typ A PVMT Line 8" | LF | \$29,260 | \$30,060 | \$17,020 | \$15,960 |
| 54572 | PVMT Symbol | EA | \$2,290 | \$2,290 | \$2,290 | \$2,290 |
| 55188 | Light Pole | EA | \$24,000 | \$24,000 | \$24,000 | \$24,000 |
| | E & S | LS | \$104,000 | \$104,000 | \$60,000 | \$60,000 |
| | Culvert | LS | \$400,000 | \$400,000 | \$400,000 | \$400,000 |
| | Utility Relocation | LS | \$50,000 | \$50,000 | \$50,000 | \$50,000 |
| | Subtotal | | \$5,949,540 | \$5,811,660 | \$3,833,310 | \$3,483,000 |
| | RR Crossing | LS | \$160,000 | \$160,000 | \$0 | \$0 |
| | Signal | per leg | \$600,000 | \$600,000 | \$540,000 | \$540,000 |
| | US 340 OR Main Street Im | provements | \$564,580 | \$564,580 | \$434,240 | \$434,240 |
| | Subtotal | | \$7,274,120 | \$7,136,240 | \$4,807,560 | \$4,457,250 |
| | Contingency | | \$2,182,240 | \$2,140,870 | \$1,442,270 | \$1,337,170 |
| | Total Construction | | \$9,456,400 | \$9,277,100 | \$6,249,800 | \$5,794,400 |

Appendix: Cost Details Cost Elements

| Item | | | | B1 | | |
|----------------------------|---------|--------------|-----------|-------------|-----------------------------|------------|
| | | | | 5,500 | | |
| 100 Mobilization | LS | 5% | _ | 280,930 | L= | 3700 |
| 111 Clearing and Grubbing | Acre | \$5,805.00 | 5.95 | 34,515 | footprint of roadway on new | location |
| 120 Regular Excavation | CY | \$21.56 | 19,185.19 | 413,633 | 70' width X 2'deep X 71% of | length |
| 505 Bedding Mat Agg 25 | Ton | \$50.00 | 13,037.04 | 651,852 | 8" X 48' footprint | \$6,519 CY |
| 580 Underdrain UD 1 | LF | \$18.55 | 11,000.00 | 204,050 | 2 X length | |
| 1080 8" Pipe | LF | \$300.00 | 550.00 | 165,000 | 10% of length | |
| 6751 Drop Inlet | EA | \$6,517.00 | 12.00 | 78,204 | 2 per 1000 feet | |
| 9056 Manhole | LF | \$846.00 | 12.00 | 10,152 | 2 per 1000 feet | |
| 10099 Aggr Mat 21 B | Ton | \$76.51 | 13,037.04 | 997,464 | 8" X 48' footprint | \$6,518.52 |
| 10611 Asphault Conc | Ton | \$102.80 | 10,320.99 | 1,060,998 | 8" X (8 + 30) X L | \$5,160.49 |
| 10637 Asphault Surface | Ton | \$131.00 | 10,320.99 | 1,352,049 | 2" X (8 + 30) X L | \$5,160.49 |
| 12600 Standard Curb Gut CG | 6 LF | \$24.99 | 300.00 | 7,497 | 300 feet 300 | |
| 13294 Guardrail GR 8 | LF | \$22.58 | 500.00 | 11,290 | 500 feet | |
| 13345 GR 9 Terminal | EA | \$2,412.00 | 6.00 | 14,472 | 6 total | |
| 27013 Topsoil | CY | \$25.00 | 2,037.04 | 50,926 | 20 feet X 6" X L | |
| 50108 Sign Panel | SF | \$28.99 | 240.00 | 6,958 | 20 SF per sign | 12 |
| 54037 Typ A PVMT Line 8" | LF | \$1.33 | 22,000.00 | 29,260 | 4 X L | |
| 54572 PVMT Symbol | EA | \$286.00 | 8.00 | 2,288 | 8 total | |
| 55188 Light Pole | EA | \$4,000.00 | 6.00 | 24,000 | 6 Total | |
| E&S | LS | | | 104,000 | | |
| Culvert | LS | | | 400,000 | | |
| Utility Relocation | LS | | | 50,000 | | |
| Subtotal | | | | 5,949,537 | | |
| RR Crossing | LS | | | 160,000 | | |
| Signal | per leg | \$200,000.00 | 3 | 600,000 | | |
| US 340 Improvements | | | | \$564,581 | | |
| Subtotal | | | | \$7,274,118 | | |
| Contingency | | 30% | | 2,182,235 | | |
| | | | | 9,456,354 | | |

Appendix: Cost Details Cost Elements

| Item | | | | | B2 | | |
|------------------------------|---------|-----|--------------|-----------|-------------|----------------------|-----------------|
| | | | | • | 5,650 | | |
| 100 Mobilization | LS | 5% | | | 274,365 | L= | 1600 |
| 111 Clearing and Grubbing | Acre | | \$5,805.00 | 2.57 | 14,926 | footprint of roadway | on new location |
| 120 Regular Excavation | CY | | \$21.56 | 8,296.30 | 178,868 | 70' width X 2'deep X | 30% of length |
| 505 Bedding Mat Agg 25 | Ton | | \$50.00 | 13,392.59 | 669,630 | 8" X 48' footprint | \$6,696 CY |
| 580 Underdrain UD 1 | LF | | \$18.55 | 11,300.00 | 209,615 | 2 X length | |
| 1080 8" Pipe | LF | | \$300.00 | 565.00 | 169,500 | 10% of length | |
| 6751 Drop Inlet | EA | | \$6,517.00 | 12.00 | 78,204 | 2 per 1000 feet | |
| 9056 Manhole | LF | | \$846.00 | 12.00 | 10,152 | 2 per 1000 feet | |
| 10099 Aggr Mat 21 B | Ton | | \$76.51 | 13,392.59 | 1,024,667 | 8" X 48' footprint | \$6,696.30 |
| 10611 Asphault Conc | Ton | | \$102.80 | 10,602.47 | 1,089,934 | 8" X (8 + 30) X L | \$5,301.23 |
| 10637 Asphault Surface | Ton | | \$131.00 | 10,602.47 | 1,388,923 | 2" X (8 + 30) X L | \$5,301.23 |
| 12600 Standard Curb Gut CG 6 | 3 LF | | \$24.99 | 300.00 | 7,497 | 300 feet 300 | |
| 13294 Guardrail GR 8 | LF | | \$22.58 | 500.00 | 11,290 | 500 feet | |
| 13345 GR 9 Terminal | EA | | \$2,412.00 | 6.00 | 14,472 | 6 total | |
| 27013 Topsoil | CY | | \$25.00 | 2,092.59 | 52,315 | 20 feet X 6" X L | |
| 50108 Sign Panel | SF | | \$28.99 | 240.00 | 6,958 | 20 SF per sign | 12 |
| 54037 Typ A PVMT Line 8" | LF | | \$1.33 | 22,600.00 | 30,058 | 4 X L | |
| 54572 PVMT Symbol | EA | | \$286.00 | 8.00 | 2,288 | 8 total | |
| 55188 Light Pole | EA | | \$4,000.00 | 6.00 | 24,000 | 6 Total | |
| E&S | LS | | | | 104,000 | | |
| Culvert | LS | | | | 400,000 | | |
| Utility Relocation | LS | | | | 50,000 | | |
| Subtotal | | | | | 5,811,661 | | |
| RR Crossing | LS | | | | 160,000 | | |
| Signal | per leg | | \$200,000.00 | 3 | 600,000 | | |
| US 340 Improvements | | | | | \$564,581 | | |
| Subtotal | | | | | \$7,136,242 | | |
| Contingency | | 30% | | | 2,140,873 | | |
| | | | | | 9,277,114 | | |

Appendix: Cost Details

Cost Elements

| Item | | | | D1 | | |
|----------------------------|---------|--------------|-----------|-------------|------------------------|----------------|
| | | | | 3,200 | | |
| 100 Mobilization | LS | 5% | | 180,158 | L= | 3000 |
| 111 Clearing and Grubbing | Acre | \$5,805.00 | 4.82 | 27,986 | footprint of roadway o | n new location |
| 120 Regular Excavation | CY | \$21.56 | 15,555.56 | 335,378 | 70' width X 2'deep X 3 | 30% of length |
| 505 Bedding Mat Agg 25 | Ton | \$50.00 | 7,585.19 | 379,259 | 8" X 48' footprint | \$3,793 CY |
| 580 Underdrain UD 1 | LF | \$18.55 | 6,400.00 | 118,720 | 2 X length | |
| 1080 8" Pipe | LF | \$300.00 | 320.00 | 96,000 | 10% of length | |
| 6751 Drop Inlet | EA | \$6,517.00 | 12.00 | 78,204 | 2 per 1000 feet | |
| 9056 Manhole | LF | \$846.00 | 12.00 | 10,152 | 2 per 1000 feet | |
| 10099 Aggr Mat 21 B | Ton | \$76.51 | 7,585.19 | 580,343 | 8" X 48' footprint | \$3,792.59 |
| 10611 Asphault Conc | Ton | \$102.80 | 6,004.94 | 617,308 | 8" X (8 + 30) X L | \$3,002.47 |
| 10637 Asphault Surface | Ton | \$131.00 | 6,004.94 | 786,647 | 2" X (8 + 30) X L | \$3,002.47 |
| 12600 Standard Curb Gut CG | 3 LF | \$24.99 | 300.00 | 7,497 | 300 feet 300 | |
| 13294 Guardrail GR 8 | LF | \$22.58 | 500.00 | 11,290 | 500 feet | |
| 13345 GR 9 Terminal | EA | \$2,412.00 | 6.00 | 14,472 | 6 total | |
| 27013 Topsoil | CY | \$25.00 | 1,185.19 | 29,630 | 20 feet X 6" X L | |
| 50108 Sign Panel | SF | \$28.99 | 240.00 | 6,958 | 20 SF per sign | 12 |
| 54037 Typ A PVMT Line 8" | LF | \$1.33 | 12,800.00 | 17,024 | 4 X L | |
| 54572 PVMT Symbol | EA | \$286.00 | 8.00 | 2,288 | 8 total | |
| 55188 Light Pole | EA | \$4,000.00 | 6.00 | 24,000 | 6 Total | |
| E&S | LS | | | 60,000 | | |
| Culvert | LS | | | 400,000 | | |
| Utility Relocation | LS | | | 50,000 | | |
| Subtotal | | | | 3,833,312 | | |
| RR Crossing | LS | | | | | |
| Signal | per leg | \$180,000.00 | 3 | 540,000 | | |
| Main Street Improvemer | nts | | | \$434,244 | | |
| Subtotal | | | | \$4,807,555 | | |
| Contingency | | 30% | | 1,442,267 | | |
| | | | | 6,249,822 | | |

Appendix: Cost Details Cost Elements

| Item | | | | | D2 | | |
|----------------------------|---------|------|--|-----------|-------------------------|----------------------|------------|
| 100 Mobilization | LS | 5% | | | 3,000 163,476 | L= | 1600 |
| 111 Clearing and Grubbing | Acre | J 70 | \$5,805.00 | 2.57 | • | footprint of roadway | |
| 120 Regular Excavation | CY | | \$21.56 | 8,296.30 | | 70' width X 2'deep X | |
| 505 Bedding Mat Agg 25 | Ton | | \$50.00 | 7,111.11 | • | 8" X 48' footprint | \$3,556 CY |
| 580 Underdrain UD 1 | LF | | \$18.55 | 6,000.00 | • | 2 X length | ψ5,550 Ο Ι |
| 1080 8" Pipe | LF | | \$300.00 | 300.00 | • | 10% of length | |
| 6751 Drop Inlet | EA | | \$6,517.00 | 12.00 | • | 2 per 1000 feet | |
| 9056 Manhole | LF | | \$846.00 | 12.00 | • | 2 per 1000 feet | |
| 10099 Aggr Mat 21 B | Ton | | \$76.51 | 7,111.11 | • | 8" X 48' footprint | \$3,555.56 |
| 10611 Asphault Conc | Ton | | \$102.80 | 5,629.63 | • | 8" X (8 + 30) X L | \$2,814.81 |
| 10637 Asphault Surface | Ton | | \$131.00 | 5,629.63 | • | 2" X (8 + 30) X L | \$2,814.81 |
| 12600 Standard Curb Gut CG | | | \$24.99 | 300.00 | • | 300 feet 300 | Ψ2,014.01 |
| 13294 Guardrail GR 8 | LF | | \$22.58 | 500.00 | , | 500 feet | |
| 13345 GR 9 Terminal | EA | | \$2,412.00 | 6.00 | • | 6 total | |
| 27013 Topsoil | CY | | \$25.00 | 1,111.11 | • | 20 feet X 6" X L | |
| 50108 Sign Panel | SF | | \$28.99 | 240.00 | , | 20 SF per sign | 12 |
| 54037 Typ A PVMT Line 8" | LF | | \$1.33 | 12,000.00 | 15,960 | | |
| 54572 PVMT Symbol | EA | | \$286.00 | 8.00 | , | 8 total | |
| 55188 Light Pole | EA | | \$4,000.00 | 6.00 | • | 6 Total | |
| E & S | LS | | V 1,000100 | 0.00 | 60,000 | | |
| Culvert | LS | | | | 400,000 | | |
| Utility Relocation | LS | | | | 50,000 | | |
| Subtotal | | | | | 3,483,003 | | |
| RR Crossing | LS | | | | .,, | | |
| Signal | per leg | | \$180,000.00 | 3 | 540,000 | | |
| Main Street Improveme | | | , , , , , , , , , , , , , , , , , , , | | \$434,244 | | |
| Subtotal | | | | | \$4,457,246 | | |
| Contingency | | 30% | | | 1,337,174 | | |
| 5 | | | | | 5,794,420 | | |

Appendix: Cost Details B1 and B2

Cost of US 340 Improvements

| • | | | | | |
|------------------------------|------|-------------|--------------|---------------------------|----------------|
| 100 Mobilization | LS | 5% | | 23,313 | |
| 111 Clearing and Grubbing | Acre | \$5,805.00 | 0.00 | 0 | |
| 120 Regular Excavation | CY | \$21.56 | 1918.52 | 41,363 10% of Mainline B1 | |
| 505 Bedding Mat Agg 25 | Ton | \$50.00 | 586.42 | 29,321 4" X | 293.2098765 CY |
| 580 Underdrain UD 1 | LF | \$18.55 | 1800.00 | 33,390 2 X length | 900 |
| 1080 8" Pipe | LF | \$300.00 | 90.00 | 27,000 10% of length | 10% |
| 6751 Drop Inlet | EA | \$6,517.00 | 2.00 | 13,034 2 per 1000 feet | |
| 9056 Manhole | LF | \$846.00 | 2.00 | 1,692 2 per 1000 feet | |
| 10099 Aggr Mat 21 B | Ton | \$76.51 | 586.42 | 44,867 4" X SF | 293.2098765 |
| 10611 Asphault Conc | Ton | \$102.80 | 586.42 | 60,284 4" X SF | 293.2098765 |
| 10637 Asphault Surface | Ton | \$131.00 | 1172.84 | 153,642 8" X SF | 586.4197531 |
| 12600 Standard Curb Gut CG 6 | LF | \$24.99 | 200.00 | 4,998 200 feet | |
| 13294 Guardrail GR 8 | LF | \$22.58 | 300.00 | 6,774 300 feet | |
| 13345 GR 9 Terminal | EA | \$2,412.00 | 4.00 | 9,648 4 total | |
| 27013 Topsoil | CY | \$25.00 | 333.33 | 8,333 20 feet X 6" X L | 9000 SF |
| 50108 Sign Panel | SF | \$28.99 | 120.00 | 3,479 20 SF per sign | |
| 54037 Typ A PVMT Line 8" | LF | \$1.33 | 5400.00 | 7,182 6 X L | |
| 54572 PVMT Symbol | EA | \$286.00 | 10.00 | 2,860 10 total | |
| 55188 Light Pole | EA | \$4,000.00 | 2.00 | 8,000 2 Total | |
| E&S | LS | 10% of Main | \$104,000.00 | 10,400 | |
| Utility Relocation | LS | | | 20,000 | |
| MOŤ | Day | \$50.00 | \$1,100.00 | 55,000 | |
| Subtotal | • | • | . , | 564,581 | |
| | | | | • | |

Appendix: Cost Details D1 and D2

Cost Elements

| 100 Mobilization | LS | 5% | | 17,369 | |
|----------------------------|------|-------------|-------------|------------------------|----------------|
| 111 Clearing and Grubbing | Acre | \$5,805.00 | 0.00 | 0 | |
| 120 Regular Excavation | CY | \$21.56 | 1555.56 | 33,538 10% of Mainline | |
| 505 Bedding Mat Agg 25 | Ton | \$50.00 | 395.06 | 19,753 4" X | 197.5308642 CY |
| 580 Underdrain UD 1 | LF | \$18.55 | 1500.00 | 27,825 2 X length | 750 |
| 1080 8" Pipe | LF | \$300.00 | 75.00 | 22,500 10% of length | 10% |
| 6751 Drop Inlet | EA | \$6,517.00 | 2.00 | 13,034 2 per 1000 feet | |
| 9056 Manhole | LF | \$846.00 | 2.00 | 1,692 2 per 1000 feet | |
| 10099 Aggr Mat 21 B | Ton | \$76.51 | 395.06 | 30,226 4" X SF | 197.5308642 |
| 10611 Asphault Conc | Ton | \$102.80 | 395.06 | 40,612 4" X SF | 197.5308642 |
| 10637 Asphault Surface | Ton | \$131.00 | 790.12 | 103,506 8" X SF | 395.0617284 |
| 12600 Standard Curb Gut CG | 6 LF | \$24.99 | 200.00 | 4,998 200 feet | |
| 13294 Guardrail GR 8 | LF | \$22.58 | 300.00 | 6,774 300 feet | |
| 13345 GR 9 Terminal | EA | \$2,412.00 | 4.00 | 9,648 4 total | |
| 27013 Topsoil | CY | \$25.00 | 277.78 | 6,944 20 feet X 6" X L | 7500 SF |
| 50108 Sign Panel | SF | \$28.99 | 120.00 | 3,479 20 SF per sign | |
| 54037 Typ A PVMT Line 8" | LF | \$1.33 | 4500.00 | 5,985 6 X L | |
| 54572 PVMT Symbol | EA | \$286.00 | 10.00 | 2,860 10 total | |
| 55188 Light Pole | EA | \$4,000.00 | 2.00 | 8,000 2 Total | |
| E&S | LS | 10% of Main | \$60,000.00 | 6,000 | |
| Utility Relocation | LS | | | 20,000 | |
| MOT | Day | \$45.00 | \$1,100.00 | 49,500 | |
| Subtotal | | | | 434,244 | |
| | | | | | |

Southeastern Collector Berryville SUMMARY OF COSTS ROW TAKES

Berryville - Right-of-way Take in SF

| Number | Description | Concept B2 | Concept B1 | Concept D2 | Concept D1 |
|--------|------------------------------|------------|------------|------------|------------|
| l | NE corner US 340- Smallwood | 2,883 | 2,883 | | |
| II | SE US 340 - Smallwood Lane | 7,557 | 7,557 | | |
| III | North side of Smallwood Lane | 2,103 | 2,103 | | |
| IV | South side of Smallwood Lane | 7,404 | 7,404 | | |
| V | North side of Smallwood Lane | 894 | 894 | | |
| VI | North side of Smallwood Lane | 9,660 | 9,660 | | |
| VII | Residence just east of RR | 11,770 | 11,770 | | |
| VIII | Milton Valley Farm Property | 0 | 0 | | |
| IX | Smallwood Property | 249,372 | 231,511 | 149,848 | 188,121 |
| Χ | Pumpernickle Press | 6,245 | 14,818 | 6,245 | 14,818 |
| XI | Timberlake Cabinet | 8,576 | 8,576 | 8,576 | 8,576 |
| XII | Water tower | 2,402 | 2,402 | 2,402 | 2,402 |
| XIII | Along Jack Enders | 10,998 | 10,998 | 10,998 | 10,998 |
| XIV | West Side, Jack Enders | 18,468 | 18,468 | 18,468 | 18,468 |
| XV | West Side, Jack Enders | 0 | 0 | 0 | 0 |
| XVI | West Side, Jack Enders | 1,030 | 1,030 | 1,030 | 1,030 |
| XVII | West Side, Jack Enders | 3,803 | 3,803 | 3,803 | 3,803 |
| | Total Less IX | 93,794 | 102,366 | 51,523 | 60,095 |
| | IX (Smallwood Property) | 249,372 | 231,511 | 149,848 | 188,121 |
| | ROW Cost | \$100,312 | \$100,148 | \$58,056 | \$70,778 |

Estimated SF Costs

| Open Space (Smallwood Property) | 10,000 \$ / acre |
|---------------------------------|------------------|
| | 0.230 \$ / SF |
| Improved Residence or Business | 20,000 \$ / acre |
| | 0.459 \$ / SF |

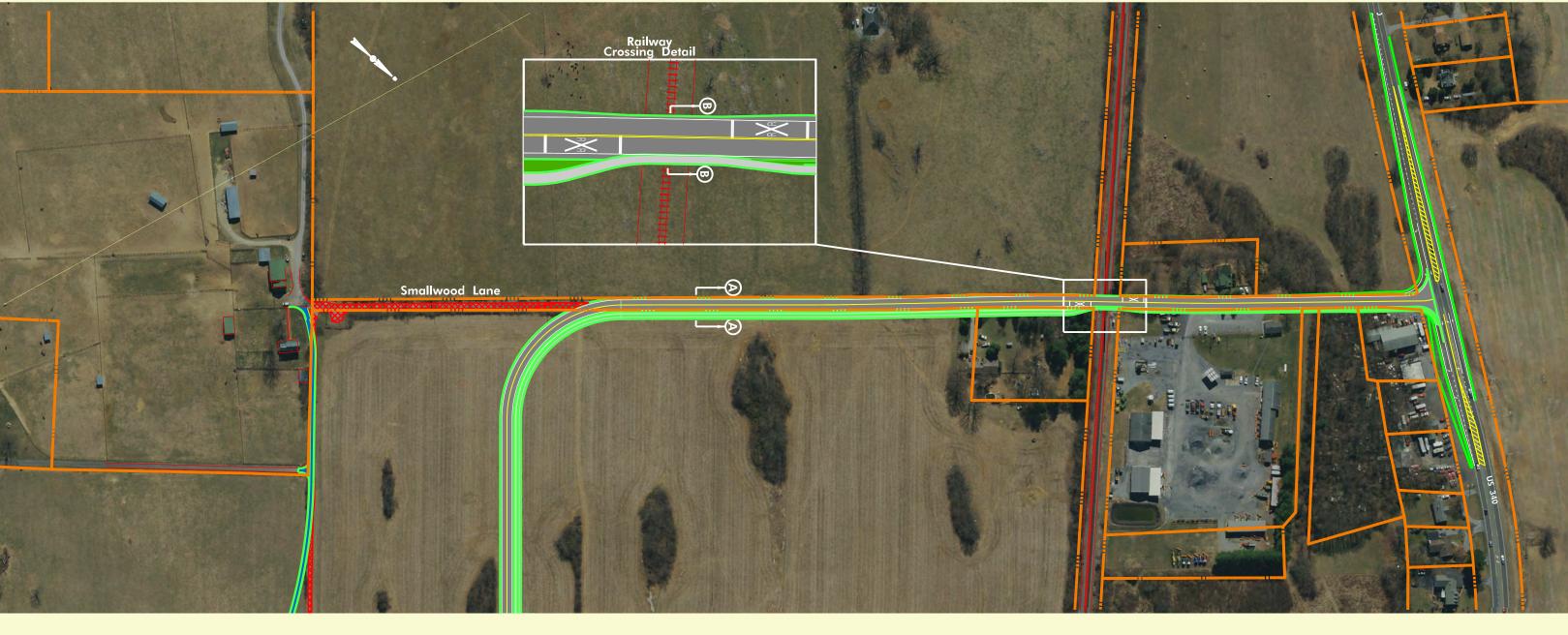
Appendix D: Conceptual Plans

Contents:

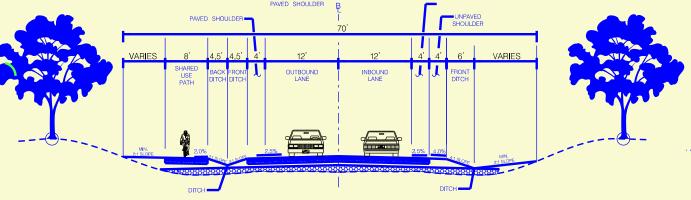
- 1. Concept B1 Part 1
- 2. Concept B1 Part 2
- 3. Concept B2 Part 1
- 4. Concept B2 Part 2
- 5. Concept D1
- 6. Concept D2
- 7. Concepts showing ROW shading
- 8. Northern Terminus (Concept B2 or D2)
- 9. Profile of Smallwood Lane
- 10. Southern Terminus (Concept B)

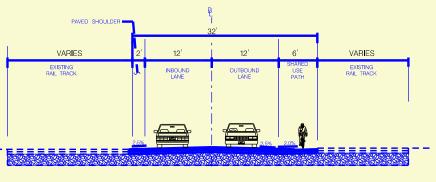


Berryville Roadway Improvements CONCEPT B1 - PART 1







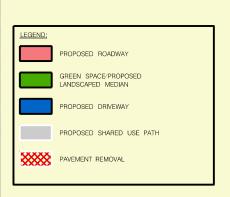


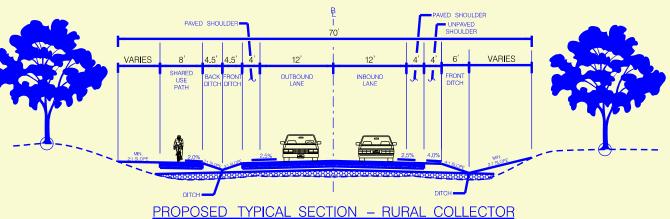
PROPOSED TYPICAL SECTION AA - RURAL COLLECTOR

PROPOSED TYPICAL SECTION BB - RURAL COLLECTOR

Berryville Roadway Improvements CONCEPT B1 - PART 2



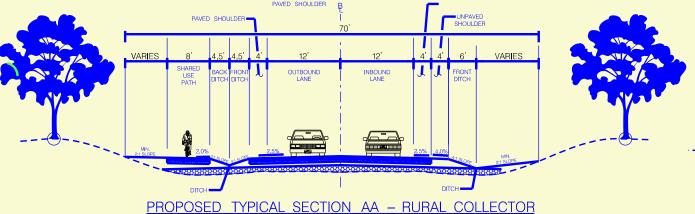


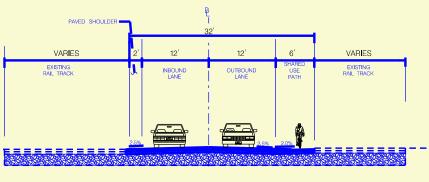


Berryville Roadway Improvements CONCEPT B2 - PART 1





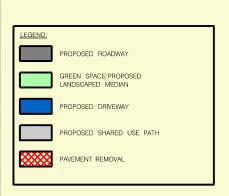


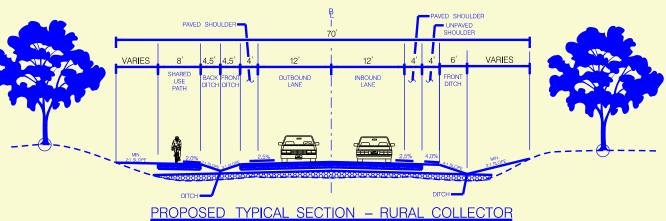


PROPOSED TYPICAL SECTION BB - RURAL COLLECTOR

Berryville Roadway Improvements CONCEPT B2 - PART 2

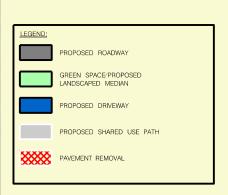


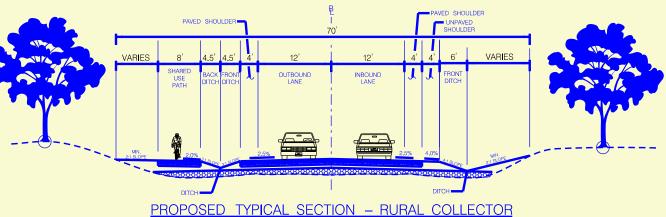




Berryville Roadway Improvements CONCEPT D1



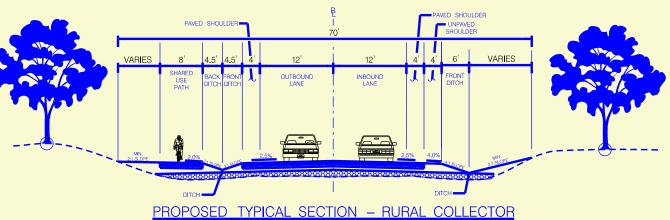




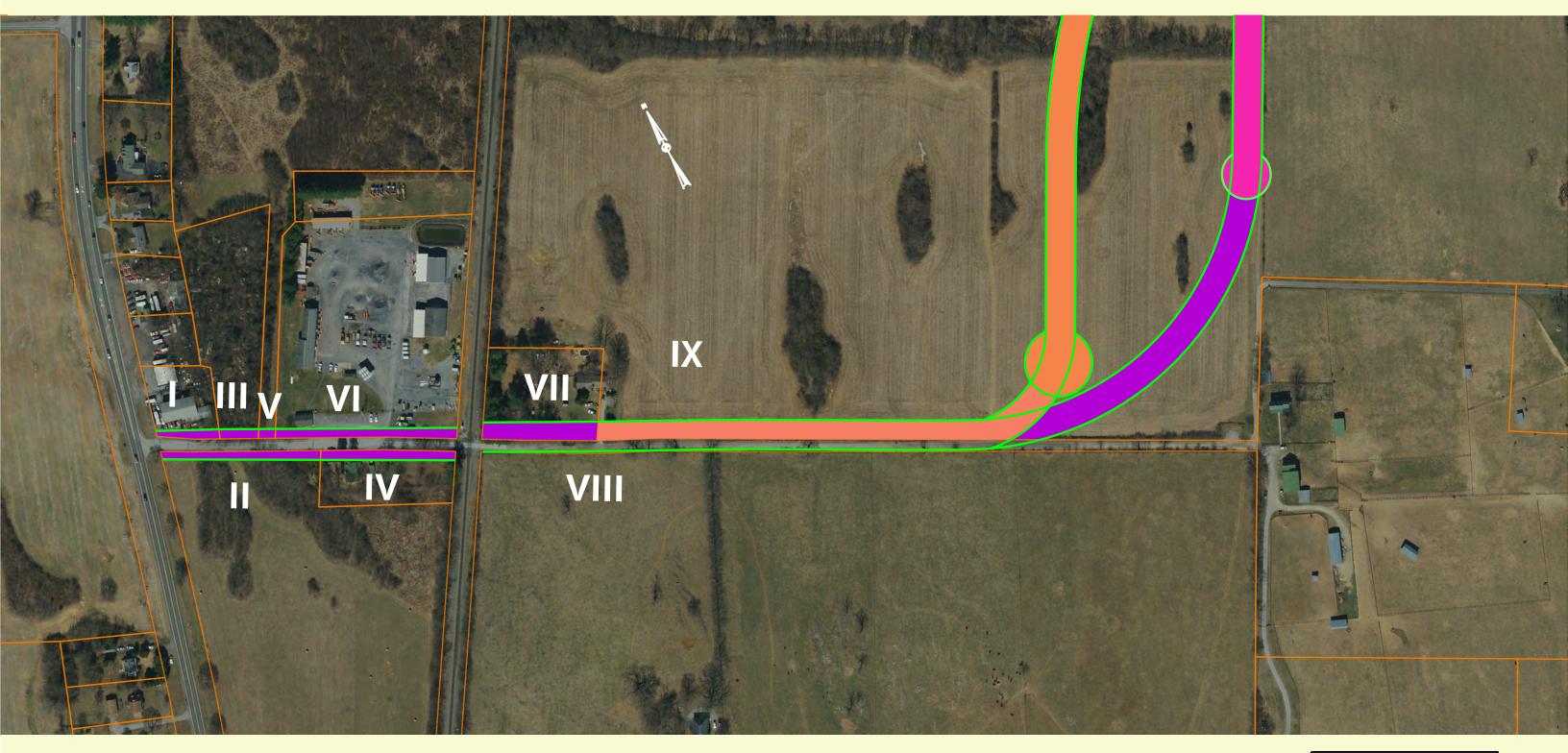
Berryville Roadway Improvements CONCEPT D2

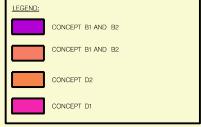




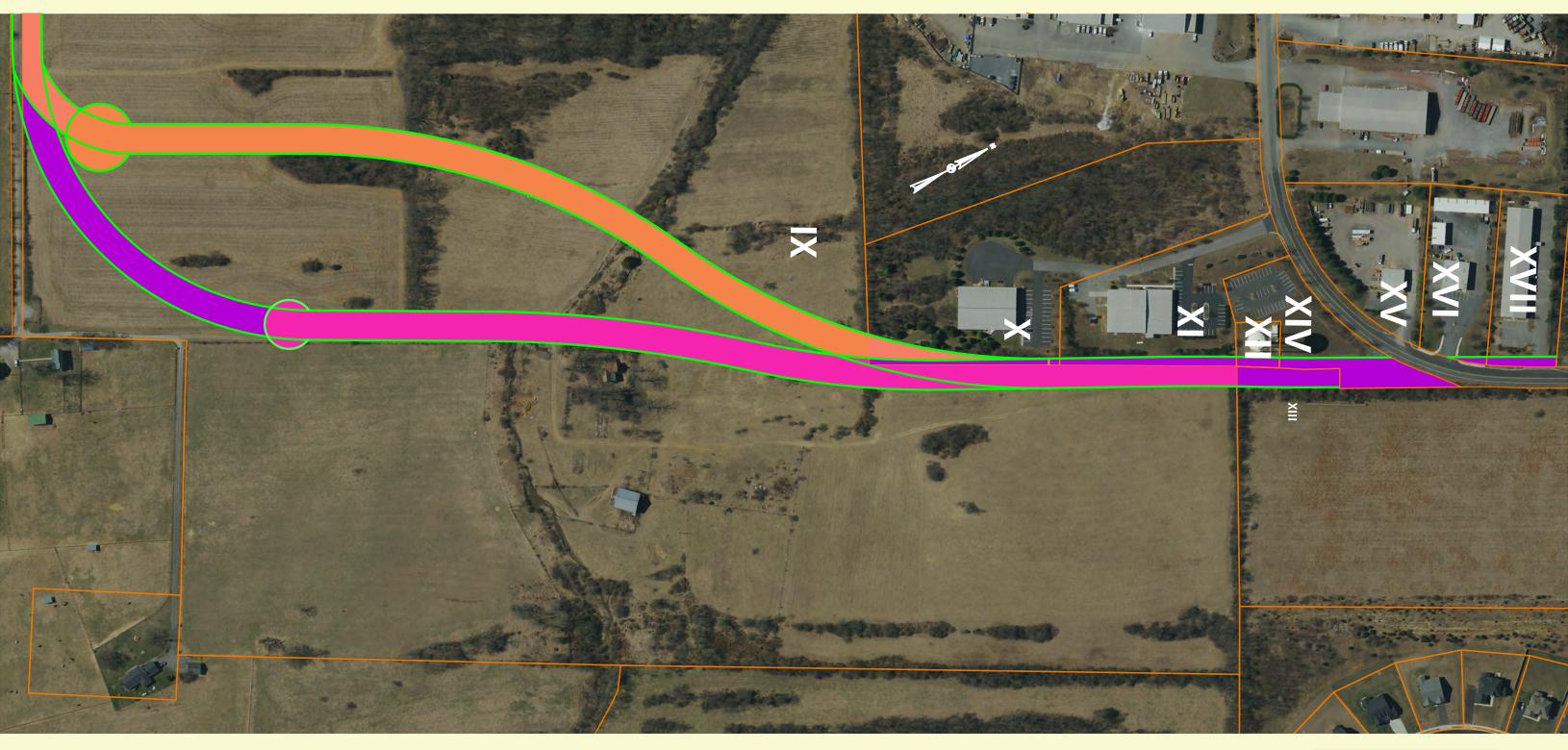


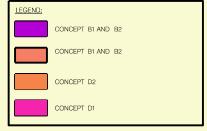
Berryville Roadway Improvements RIGHT-OF-WAY TAKING - PART 1





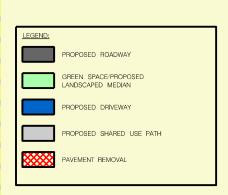
Berryville Roadway Improvements RIGHT-OF-WAY TAKING - PART 2

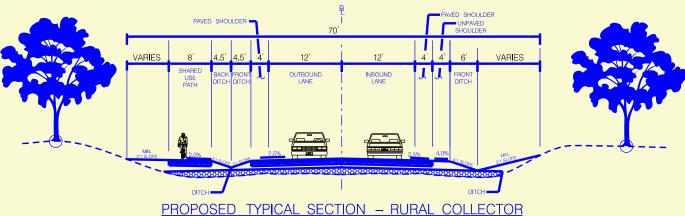




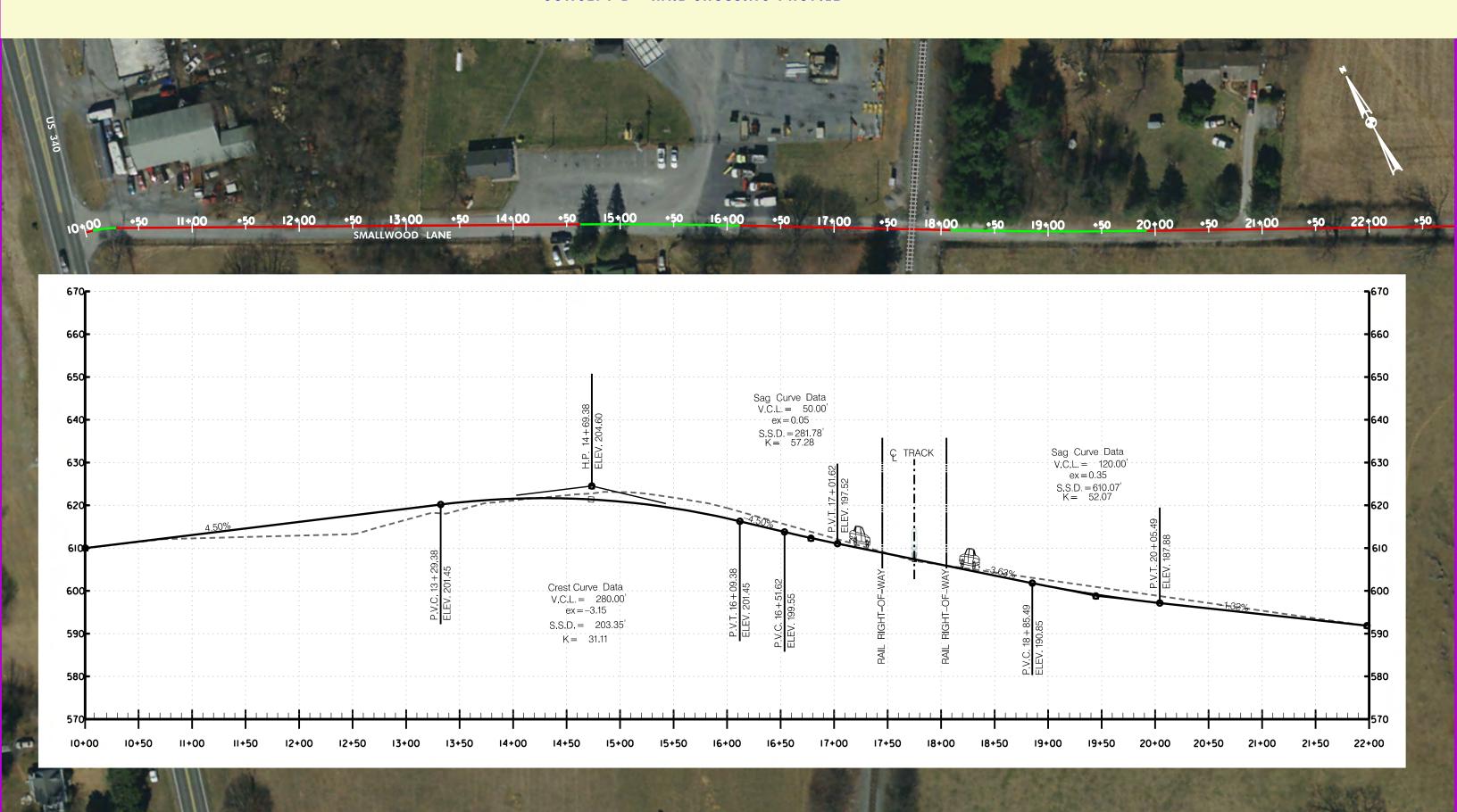
Berryville Roadway Improvements Alternate 1 CONCEPT B - NORTHERN TERMINIS







Berryville Roadway Improvements CONCEPT B - RAIL CROSSING PROFILE



Berryville Roadway Improvements CONCEPT B - SOUTHERN TERMINIS

